



Hydrogeological Assessment

Flato Southeast (Eco Park), Dundalk, Ontario

Flato Eco Park Dundalk Inc.

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1.0 Introduction

SLR Consulting (Canada) Ltd. (SLR) was retained by Flato Eco Park Dundalk Inc. to conduct a Hydrogeological Assessment as part of a Draft Plan of Subdivision and future Site Plan for the proposed Flato Southeast Eco Park residential subdivision located in Dundalk, Ontario (referred to as the "Study Area" or "the Site") (**Figure 1**).

The Site includes the properties located on lands legally identified as Part of lots 238, 239 and 240 Concession 1, Southwest of Toronto and Sydenham Road (SWTSR) and part of lots 238 and 239 Concession 2, SWTSR located in Dundalk, Ontario in support of proposals for residential development within these properties ("Site", **Figure 2**).

The Site is bounded by Highway 10 to the northeast, and Grey County Trail to the southeast. These lands fall within a larger area currently subject to an approved Ministerial Zoning Order (MZO), whereby the development of these subject lands will be phased.

1.1 Study Objectives

The objective of the Hydrogeological Assessment is to characterize the hydrogeological conditions across the Site, identify hydrogeological constraints to development and potential impacts of development on natural heritage features, and provide guidance on how to mitigate these impacts. This is completed through a review of relevant geologic and hydrogeologic information available through public records for the area or collected through borehole drilling and groundwater monitoring and sampling efforts. This report has been prepared for submission to the Township of Southgate, Bruce County, and Grand River Conservation Authority (GRCA) as part of the Draft Plan of Subdivision and future Site Plan Application for the proposed development.

The specific objectives are summarized below:

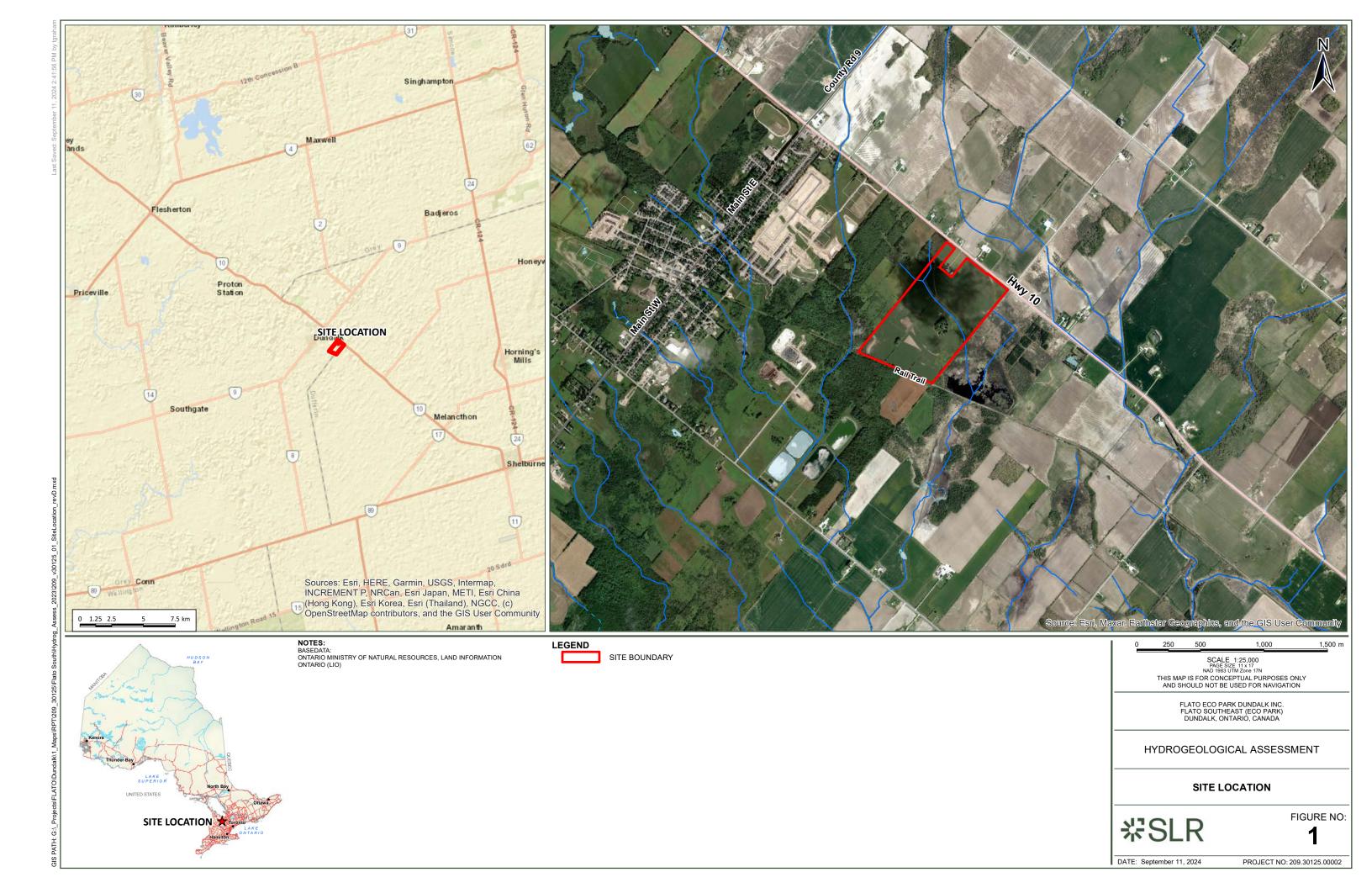
- Document the geology, hydrostratigraphy, groundwater flow, and groundwater quality across the Study Area.
- Evaluate potential impacts with respect to Source Protection Plans.
- Assess overall potential impacts of the proposed development on the groundwater flow system.

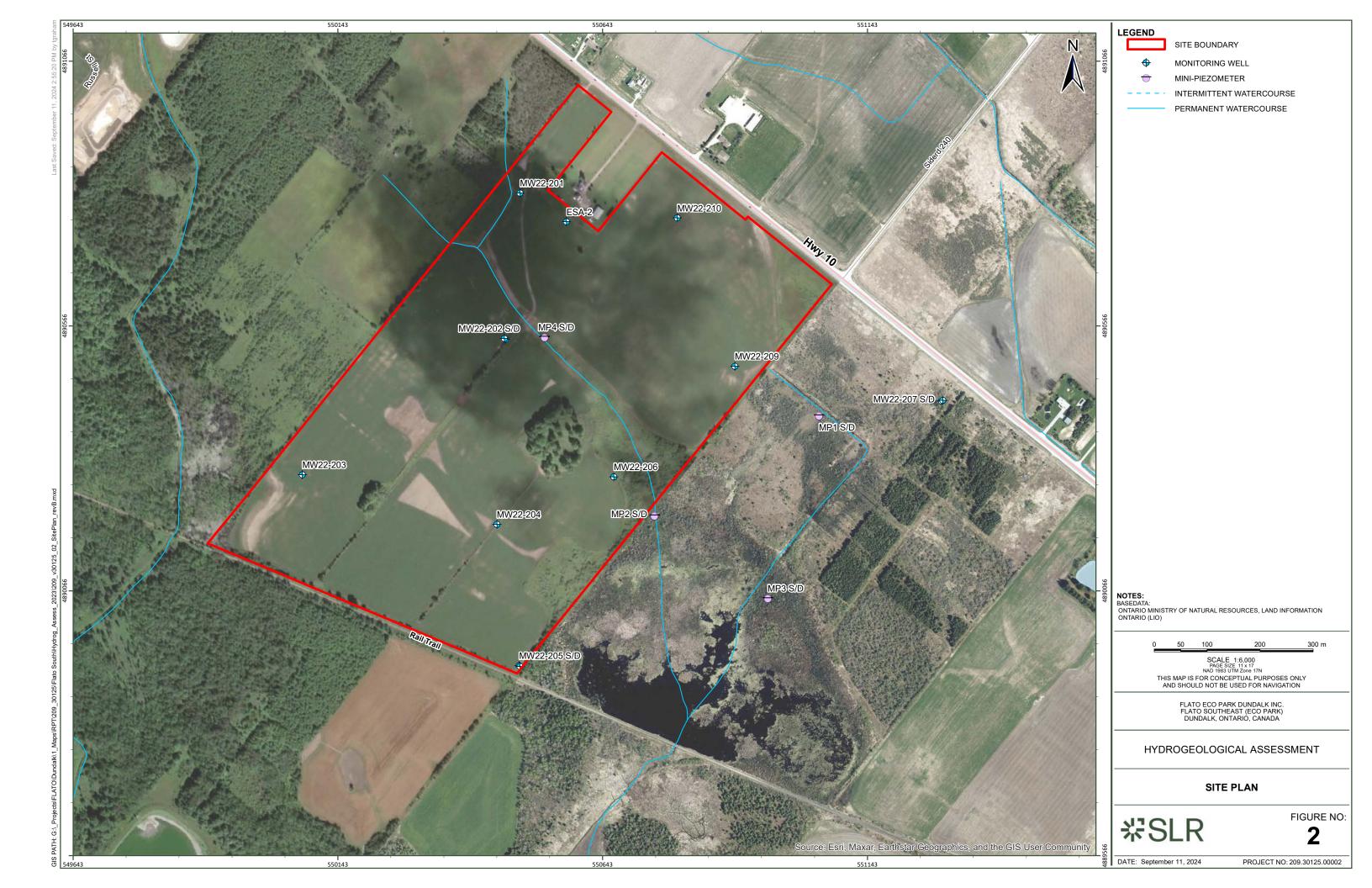
1.2 Report Organization

This Hydrogeology Assessment report has been organized into eight sections following this introduction. Section 2 provides an overview of background information related to the development, previous investigations, and regional geology and hydrogeology. Section 3 provides the field methodologies utilized during the assessment. Section 4 presents a review of the Site geological and hydrogeological conditions. Section 5 provides an assessment of the potential impacts of development on shallow groundwater features, potable wells, and surface water features. Section 6 presents the conclusions and recommendations, Section 7 provides closing comments, and Section 8 presents the report references.

All Figures referenced throughout the report are presented within the text. Appendices A through E present the: Development Plan; Borehole Logs; Groundwater Data; Hydraulic Conductivity Analyses; and MECP Water Well Records.







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2.0 Background

2.1 Proposed Development

The proposed Site is generally rectangular in shape and measures approximately 60.4 ha in size. It is understood that the proposed Flato Southeast Eco Park residential subdivision will contain single detached lots, townhouses, an apartment complex, commercial and industrial lots, a stormwater management (SWM) pond, parks, and areas of environmental protection. The overall development is expected to have completed municipal servicing, and paved access / site roadways. A copy of the proposed development plan is provided in **Appendix A**.

2.2 Site Description

The Site lies on lands legally identified as Part of lots 238, 239 and 240 Concession 1, Southwest of Toronto and Sydenham Road (SWTSR) and part of lots 238 and 239 Concession 2, SWTSR located in Dundalk, Ontario. The Site is used primarily for agricultural purposes. The area surrounding the Site is also primarily used for agricultural purposes, with scattered rural residences. The property is bounded by Highway 10 to the northeast, and Grey County Trail to the southeast (**Figure 2**).

2.3 Regional setting

2.3.1 Topography and Drainage

The Study Area is gently undulating with a gentle decrease in ground surface elevation from northwest to southeast. A topographic high of 515 metres above sea level (masl) is located near the northwestern portion of the Site, with a topographic low of 508 masl along the eastern portion of the Site. It is noted that the portion of the Site located on Part of Lot 242 Concession 1 is primarily dominated by the Melancthon Wetland Complex #1 (**Figure 3**). Ground surface within the Melancthon Wetland Complex #1 ranges from 508 masl to 510 masl.

The Site is located within the Upper Grand River Watershed, which is under the jurisdiction of the GRCA. There is an unnamed drainage feature present across the Site. The drainage feature generally extends across the property in a northwest to southeast alignment and is mapped to connect within the Melancthon Wetland Complex #1 along the southeast corner of the Site. Surface water within the drainage feature flows across the Site in a south to southeasterly direction.

2.3.2 Physiography

The Study Area lies within the Dundalk Till Plain physiographic region of Southern Ontario (Chapman and Putnam, 1984). The Dundalk Till Plain is a gently undulating, partially drumlinized and fluted surface, where the long axis of the drumlins are oriented in a southeastward direction. The Dundalk Till Plain supports extensive wetland complexes due to the presence of poorly drained depressions.

2.3.3 Regional Hydrostratigraphy

Surficial geology in the Dundalk area mainly consists of drumlinized till plains (Chapman and Putnam, 1984) comprised of the Elma Till (stony sandy silt to silt) and Catfish Creek Till (clayey silt and gravel, **Figure 4**). There are isolated deposits of glaciolacustrine, glaciofluvial icecontact and glaciofluvial outwash materials at surface and interbedded within the till plain.



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These sand and gravel deposits form the Dundalk Aquifer (Saugeen Valley Source Protection Area, 2015). The extent and thickness of the Dundalk Aquifer is unknown due to a lack of reliable well records for the area. It is noted that static water levels within the Dundalk Aquifer are close to ground surface.

The overburden material is underlain by bedrock aquifer units comprised of the Guelph, Eramosa, Goat Island and Gasport Formations (Golder, 2018).

2.3.4 Source Protection

Source Protection Plans (SPPs) have been implemented throughout the region to protect drinking water resources, as mandated by the Ontario Clean Water Act (OCWA), 2006. The susceptibility of an aquifer to contamination is evaluated to identify the most vulnerable areas surrounding a drinking water source. There are four (4) types of vulnerable areas as defined by the Clean Water Act, 2006:

- Highly vulnerable aquifer (HVA): aquifers in which an external source is likely to have a significant adverse effect, this includes the land above the aquifer;
- Significant groundwater recharge area (SGRA): an area in which it is necessary to regulate or monitor drinking water threats that could affect the recharge of an aquifer;
- Surface water intake protection zone (IPZ): an area related to a surface water intake area in which it is necessary to regulate or monitor drinking water threats; and
- Wellhead protection area (WHPA): an area related to a wellhead, within which it is necessary to regulate or monitor drinking water threats.

The Site is within the Grand River Source Protection Region. The Approved Source Protection Plan (GRCA, 2022) have identified the Site to be located immediately east of a WHPA-D, representing a capture zone time frame of 25 years (**Figure 5**). It is noted that a small sliver of the southwest corner of the property lies within the WHPA-D. The property is also located within a SGRA, IPZ, and HVA (**Figure 6**).

Groundwater and surface water resources within a WHPA, SGRA, IPZ, and HVA are relatively sensitive to chemical or pathogen contamination and / or changes in groundwater recharge. It is important to note that delineation of the vulnerable areas based on regional mapping and do not consider site-specific conditions (i.e., type and thickness of the overlying material). The results of the drilling program indicates that the subsurface soils across the Study Area consists of mainly silty sand till. The material was determined to have low hydraulic conductivity and therefore, the potential to impact deeper aguifers is limited.

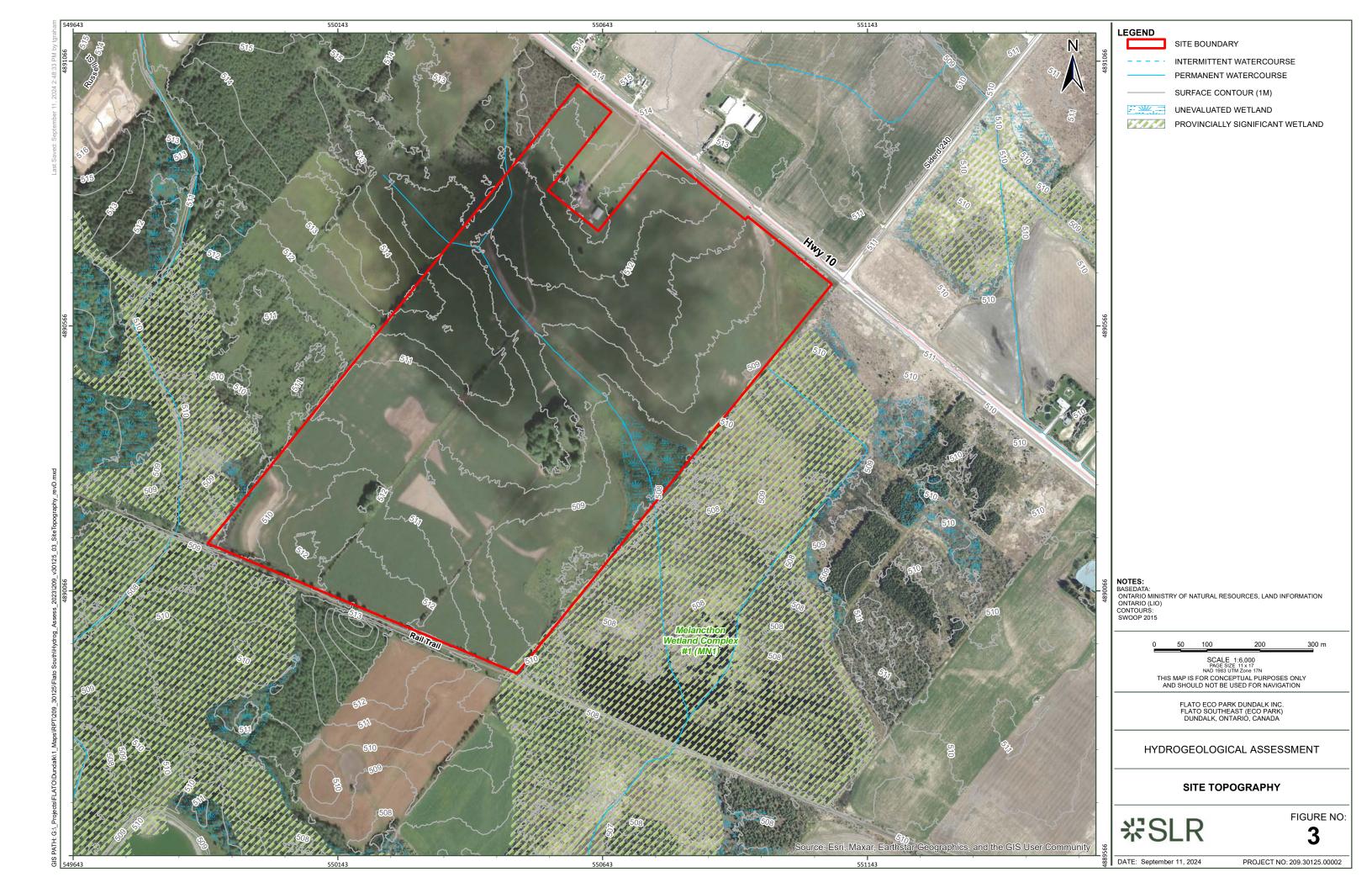
Although precautionary measures to protect groundwater and surface water must be applied on all projects, additional protection measures and related documentation may be required where study areas fall within these zones. These include maintenance of the Site-wide water balance and limitations on the presence of potential contamination sources such as gas stations and dry cleaner facilities. Based on the current development plan, the east portion of the Site includes industrial development. The Township of Southgate Official Plan (2022) encourages industrial land uses to be located in a designated industrial area (ie. Eco Park Business Park). It is further stated in the official plan that separation distances shall be provided between incompatible land uses in accordance with the D-6 guidelines of the Ministry of the Environment, Conservation and Parks. A D-6 Study is being completed for the Flato Eco Park development and will be submitted under separate cover.

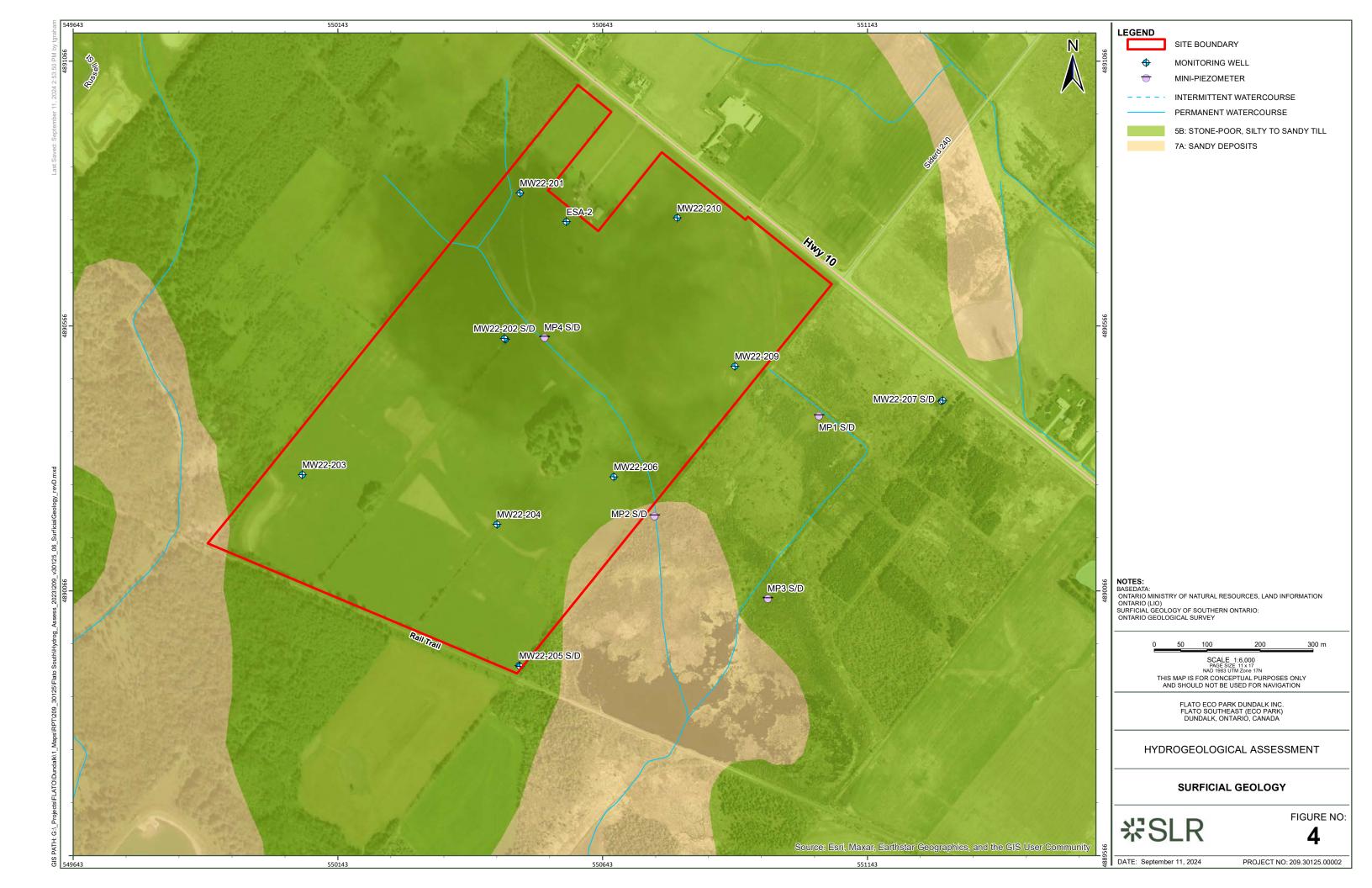


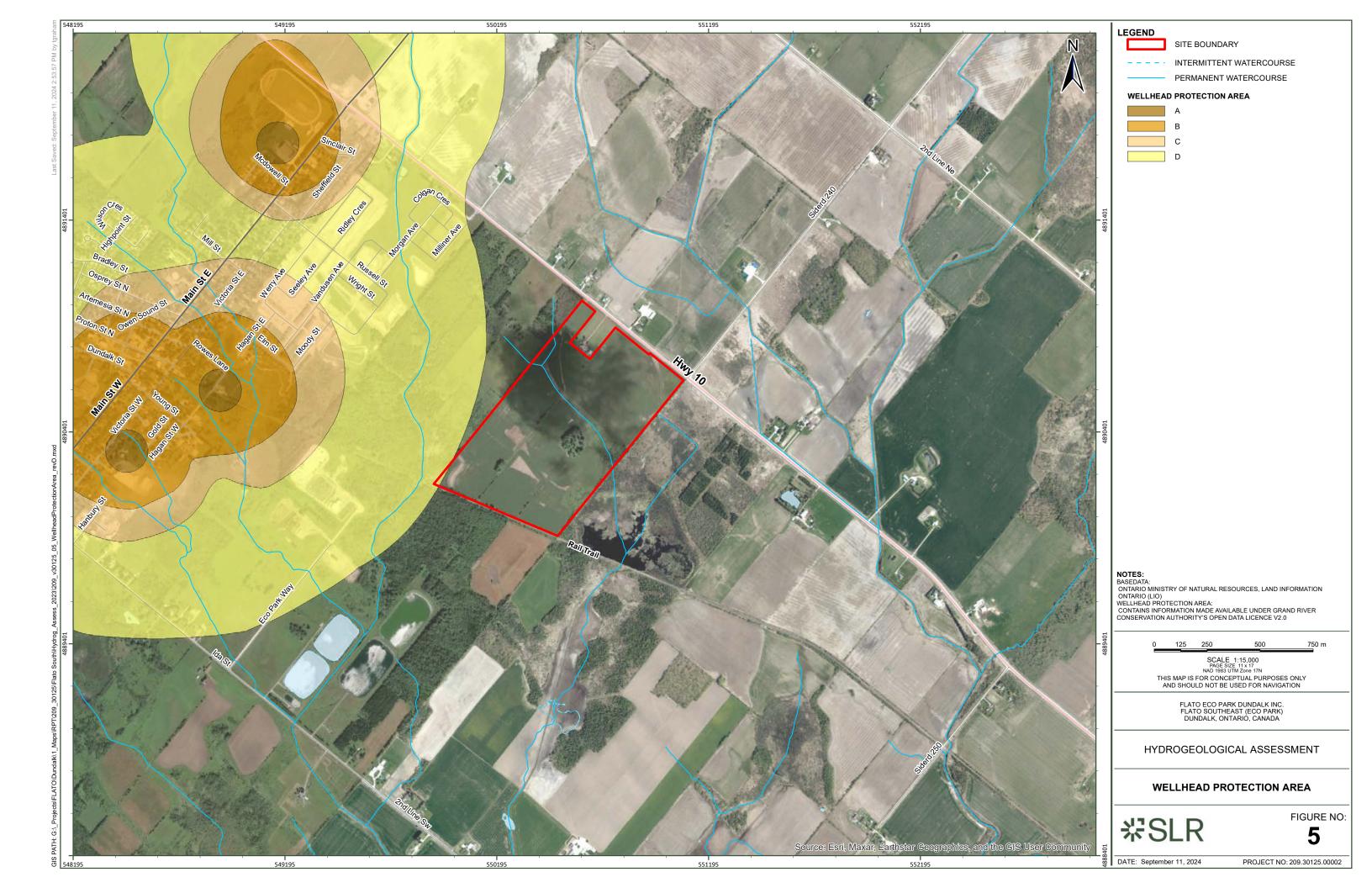
A site-specific water balance has been completed by Crozier & Associates Consulting Engineers (Crozier) to document pre-development recharge rates, and to look for opportunities to promote the recharge of clean water to meet or exceed pre-development recharge rates. The site-specific water balance is presented under separate cover.

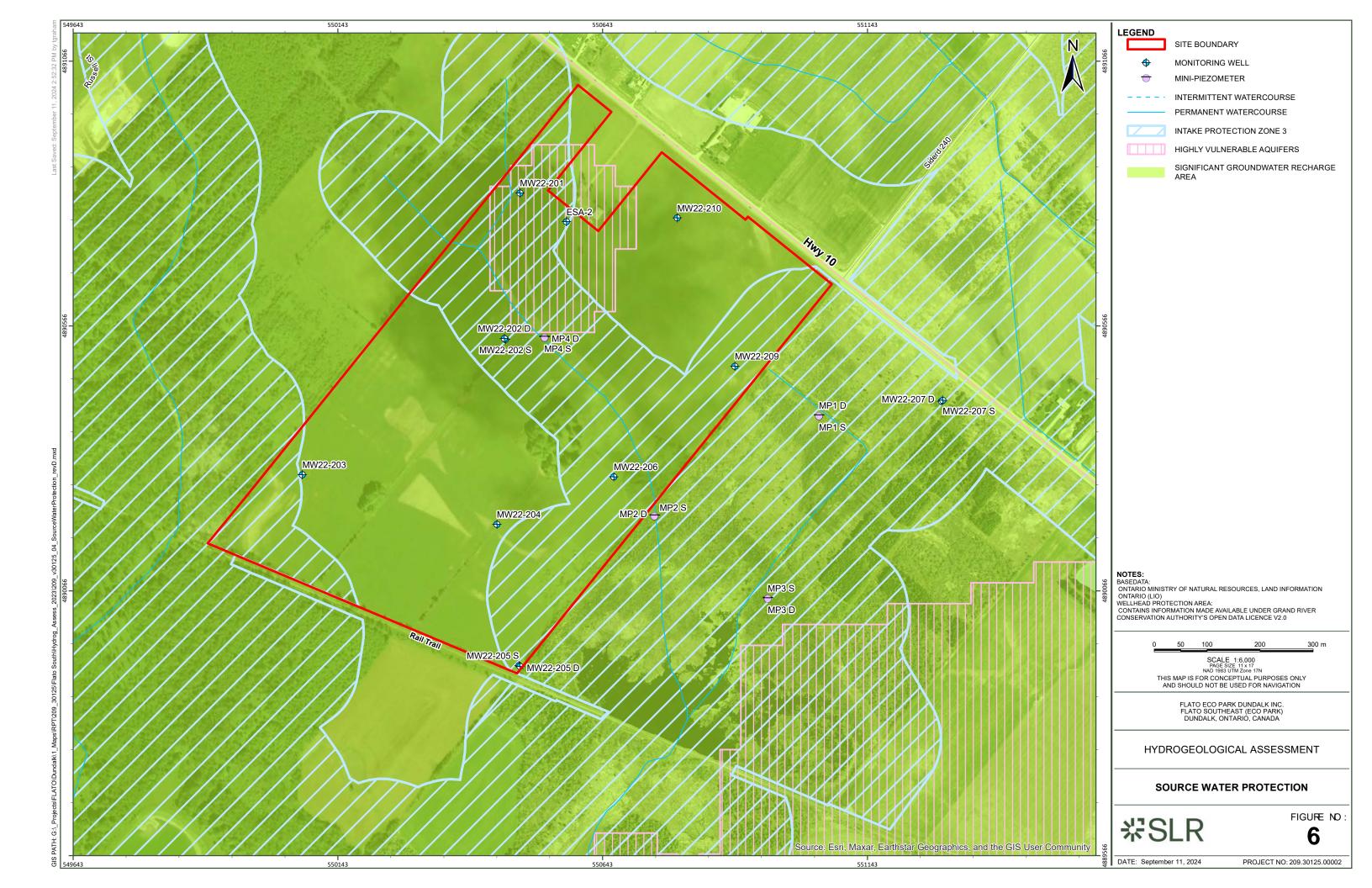
Although precautionary measures to protect groundwater and surface water resources must be applied on all projects, additional protection measures (i.e., best management plans) and related documentation may be required where subject properties fall within these zones. These include limitations on the presence of potential contamination sources such as bulk fuel or bulk chemicals. Although not anticipated to be part of the proposed development, should a bulk fuel (≥ 2,500 L) facility be included as part of the development, a Risk Management Plan (RMP) and/or Contaminant Management Plan (CMP) will be prepared, as required under the GRCA SPP (2022) Policy GC-S-CW-7.2.











3.0 Methodology

3.1 Installation of New Monitors

Nine (9) boreholes were advanced at select locations across the Site in April 2022. The boreholes were drilled using a track-mounted drill rig with 9" outer diameter hollow stem auger. A record of geological and hydrogeological conditions was logged during drilling using a split spoon sampler at approximately 0.76 m intervals down to the targeted depth of the monitoring well. At each borehole location, the soil stratigraphy and classification, moisture content, colour, appearance, soil structure (presence of laminations, heterogeneity, soil weathering, etc.), and odour was noted in general accordance with the Unified Soil Classification System. It is noted that a monitoring well (MW22-208) was proposed for the wetland area, however, was terminated due to accessibility issues.

All borehole locations were completed as monitoring wells. At three (3) of these locations nested monitoring wells, consisting of a shallow and deep counterpart, were installed. The monitoring wells were constructed with a 50-millimetre (mm) diameter polyvinyl chloride (PVC) well pipe. In general, the monitoring wells were constructed with No. 10 slotted PVC screen approximately 1.5 m long. Monitor ESA-2 was installed near a potential tank in the vicinity of the residential property as part of a Phase 2 Environmental Site Assessment completed coincident with the additional drilling program in 2022. A longer screen (3.0 m) was installed at this location to allow for an assessment of potential petroleum impacts.

A sand pack was placed around and slightly above the well screen, and the remaining upper portion of the borehole was sealed with bentonite. Monitor MW22-205D was sealed with grout as flowing sands were encountered during drilling. A steel monument casing was installed over the well at each monitoring location. Upon completion of the monitoring wells, the monitors were tagged registered with the MECP as required by Ontario Regulation (O. Reg.) 903, as amended. Details of the monitoring well construction are summarized in **Table 3-1**. The location of the monitoring wells are depicted in **Figure 7**, and borehole logs are provided in **Appendix B**.

Four (4) nested pairs of piezometers, for a total of eight (8) mini-piezometers (MP1-S/D through MP4-S/D) were installed within the tributaries and Melancthon Wetland Complex #1 in May 2022. These mini-piezometers were installed to assess groundwater-surface water interactions within the natural heritage features. The mini-piezometers were constructed with a 19 mm diameter steel pipe threaded onto an approximately 0.33 m long screened drive point piezometer Solinst tip, and were installed to the targeted depth through direct push. A pilot hole was not advanced prior to the installation; as such, the screened material at each mini-piezometer location is unknown. The construction details of the mini-piezometers are provided in **Table 3-1**, and the location of the mini-piezometers are shown on **Figure 7**.



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Table 3-1: Monitoring Well Details

Monitor	Ground Surface Elevation (masl)	Top of Pipe Elevation (masl)	Screen Interval (masl)	Screened Material
MW22-201	511.6	512.4	507.0 - 505.5	Silty sand till
MW22-202 S	510.9	511.9	506.6 – 505.1	Silty sand, Silty sand till
MW22-202 D	511.0	511.8	501.8 – 500.3	Silty sand till
MW22-203	510.4	511.3	505.8 - 504.3	Silty sand till
MW22-204	508.7	509.6	504.3 – 502.8	Silty sand till
MW22-205 S	509.4	510.3	505.0 - 503.5	Silty sand, sand and gravel
MW22-205 D	509.3	510.3	500.2 – 498.7	Sandy silt till
MW22-206	508.5	509.4	507.1 – 505.6	Silty sand till
MW22-207 S	510.1	510.9	508.6 – 507.1	Silty sand till
MW22-207 D	510.1	511.0	506.1 – 504.6	Silty sand till
MW22-209	509.1	510.0	505.5 - 503.9	Silty sand till
MW22-210	511.9	513.0	507.5 – 506.0	Silty sand till
ESA-2	512.4	513.3	511.1 – 508.0	Silty sand till
MP1-S	508.9	509.4	508.2 – 507.9	-
MP1-D	508.8	509.7	507.3 – 506.9	-
MP2-S	507.5	508.2	507.0 - 506.7	-
MP2-D	507.5	508.3	506.0 - 505.7	-
MP3-S	507.4	508.1	506.8 - 506.5	-
MP3-D	507.4	508.3	506.0 - 505.6	-
MP4-S	509.6	510.2	508.4 – 508.1	-
MP4-D	509.6	510.4	508.0 - 507.7	-

3.2 **Monitoring Well Development**

Following installation, the monitoring wells were developed using dedicated tubing with a submersible pump. The monitoring wells were developed to remove any soil fines that may have infiltrated into the monitoring well and its surrounding sand pack during the installation process, and to improve the hydraulic connection between the well and geologic materials. Due to slow recovery, each well was purged dry and allowed to recover. Water was subsequently removed from the monitoring well until discontinuous flow was produced for a second time.

3.3 Water Level Monitoring

Groundwater levels were manually collected in each accessible monitor using a water level meter to collect baseline data prior to Site development. Water levels were collected on a quarterly basis commencing on May 13, 2022, with the most recent event occurring on



June 12, 2024. The surface water level and groundwater elevation were measured at the minipiezometer locations to assess groundwater-surface water interactions within the natural heritage features.

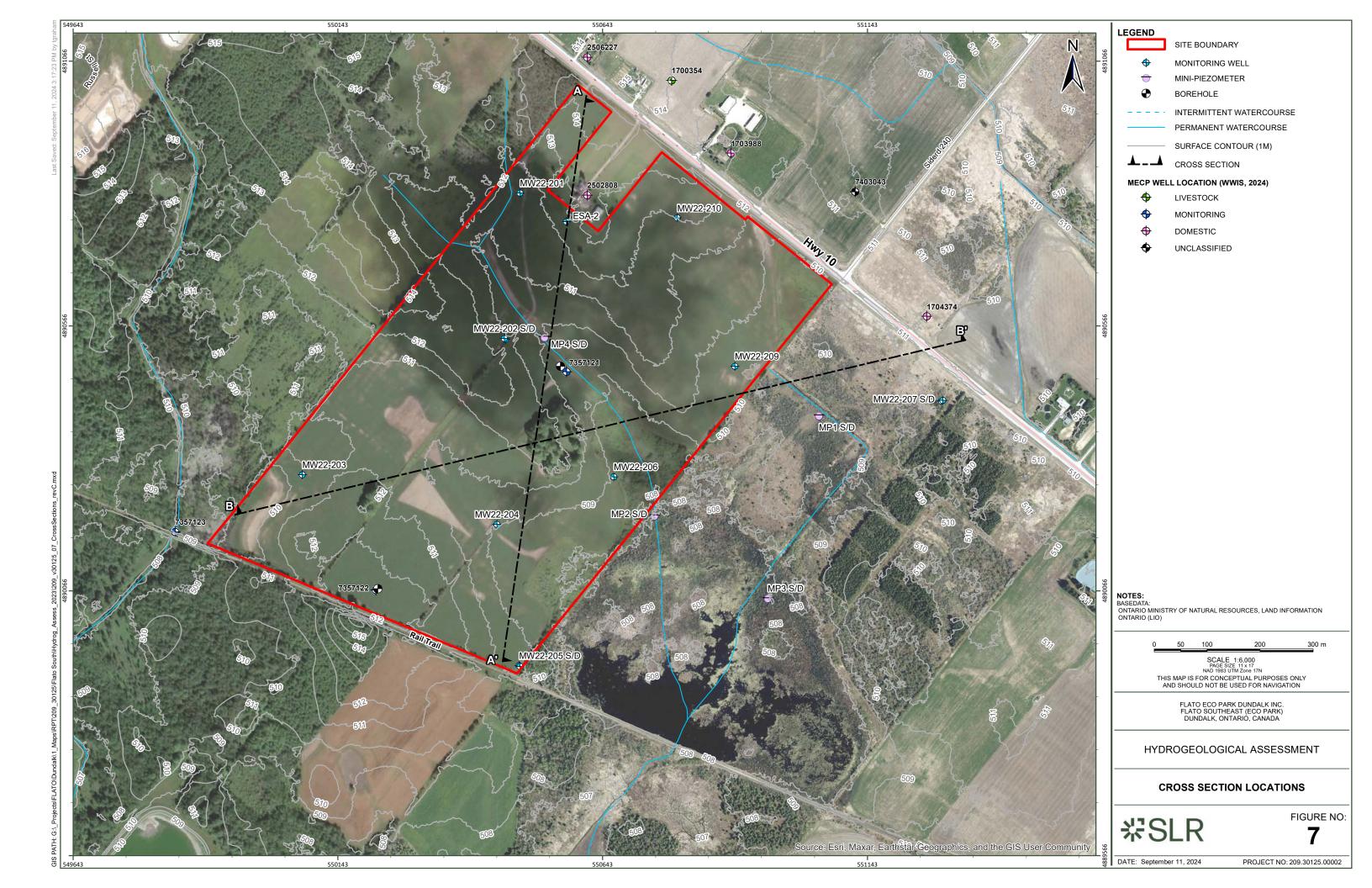
To support a more comprehensive understanding of the Site, select monitoring wells and minipiezometers were instrumented with automated dataloggers in order to obtain continuous groundwater level readings. A barologger was also deployed coincident with the datalogger to measure changes in atmospheric pressure. Continuous water level measurements provide additional insight into the groundwater regime, particularly in response to precipitation events, as well as high-water level conditions. The dataloggers are downloaded every four (4) months while completing manual water level measurements on-Site. The dataloggers were removed from the mini-piezometers during the winter period to avoid minimize potential damage due to freeze-thaw events. The dataloggers were re-deployed in the mini-piezometers in spring. All dataloggers were removed from groundwater wells and mini-piezometers on November 1, 2023, and reinstalled on March 7, 2024.

3.4 In-Situ Hydraulic Conductivity and Analysis

In-situ hydraulic conductivity tests were completed in select monitoring wells to establish the permeability (hydraulic conductivity) of the formation in which the wells are screened. Hydraulic conductivity is a parameter that describes the ability of soil to allow water to move through it. The lower the hydraulic conductivity, the less water will be able to move through. Aquifers, such as sandy or gravelly soils, typically have a hydraulic conductivity of 10⁻⁶ metres per second (m/s) or greater, whereas aquitards (clay or dense silt) have a hydraulic conductivity of 10⁻⁸ m/s or less.

The testing involved the slug test method, whereby a slug of known volume was removed (rising head test) from each well. The water levels were recorded during the addition, removal, and recovery stages of the slug test using a Diver datalogger temporarily installed in the monitor. The in-situ hydraulic conductivity test was completed once the water level recovered to 90% of static conditions. The slug tests were analyzed in AQTESOLV using the Hvorslev method (1951) for confined aguifers.





4.0 Site Geology and Hydrogeology

4.1 Geology and Hydrostratigraphy

4.1.1 Surficial Geology

Based on a review of the Ontario Geological Survey mapping (OGS, 2010), the surficial geology of the Site is primarily Elma Till, which is characterized as a stone-poor sandy silt to silty sand till. The wetland found along the western portion of the Site is mapped to consist of sandy glaciofluvial sandy deposits, with minor organic deposits located within wetland areas.

Surficial geology of the Site was also characterized by advancing boreholes at select locations across the property. Borehole logs are provided in **Appendix B**. Geological cross-sections of the Site, as indicated in **Figure 7**, are presented in **Figure 8** and **Figure 9**.

Based on the results of the drilling program, the Site was comprised of a till unit underlying the surficial, overturned topsoil. The till unit is composed of sandy silt to silty sand with gravel material. At select locations across the Site, thin (<1 m thick) surficial deposits comprised of silty sand as well as discontinuous sand/gravel interbeds was encountered. Interbedded sand to sandy gravel material were located at approximately 503.9 to 504.5 masl, and 506.9 to 508.09 masl. The glacial till material serves as an aquitard protecting the underlying bedrock aquifer due to its low permeability and substantial thickness.

4.1.2 Bedrock Geology

Boreholes advanced across the Site were terminated once the targeted depth of the shallow monitoring wells were reached. As such, bedrock was not encountered during drilling. However, a review of the MECP WWR database indicates that the bedrock in the vicinity of the Site lies between 20 mbgs (MECP well ID 2502808) to 46 mbgs (MECP well ID 1703988). The bedrock consists mostly of dolostone/limestone, likely from the Guelph Formation.

Source Protection documents from the Grand River Conservation Authority indicates that the bedrock is composed of 88 m of both the Guelph Formation and the Gasport Formation (Lake Erie Region Source Protection Committee, 2021). The Guelph Formation consists of porous, fine to medium crystalline, medium to massive irregularly bedded dolostone (Armstrong, 2010). The underlying Gasport Formation consists of thick- to massive-bedded, fine to coarse-grained dolostone and dolomitic limestone (Armstrong, 2010).

4.2 Groundwater Monitoring

4.2.1 Groundwater Monitoring

Groundwater level measurements were recorded at each accessible monitoring well and minipiezometer location commencing in May 2022 with the most recent event occurring in June 2024. Monitors MW22-202 S/D, MW22-203, MW22-204, and MW22-209, as well as minipiezometers MP1 S/D, MP2 S/D, and MP4 S/D were instrumented with dataloggers to collect continuous water level measurements. It was noted that MP4D became damaged (bent) during the monitoring program and therefore manual water level measurements are presented, but not used in vertical gradient calculations. MP3, located off site, was removed from the monitoring program due to accessibility issues. Groundwater elevations and hydrographs are provided in **Appendix C**.



Groundwater elevations across the Site fluctuated seasonally between May 2022 and June 2024. During the May 2022, March 2023, and March 2024 (spring) monitoring events, water levels in the monitoring wells ranged between 506.44 masl (MW22-209) and 512.29 masl (ESA-2). In comparison, water levels during the July 2022 (summer) event ranged between 506.82 masl (MW22-206) and 511.14 masl (MW22-210). During the November 2022 and 2023 (fall) monitoring events, groundwater levels ranged between 506.26 masl (MW22-204) and 510.72 masl (ESA-2). Groundwater elevations subtly followed surface topography, where the highest and lowest water levels were often observed at ESA-2 (northern portion of the Site) and MW22-206 (southern portion of the Site), respectively.

Groundwater elevations between the shallow and deep monitors at the nested monitoring well locations are comparable, although consistently slightly higher in the shallow monitors.

Noted periods of drawdown in all monitoring wells with dataloggers occurred between May and October of the 2022 and 2023 monitoring seasons. Water levels increased slightly during the later half of July 2023 then resumed drawdown patterns observed previously in 2022 until October 2023. Water level gradually increased from October to peak levels in March during both the 2023 and 2024 monitoring seasons.

Groundwater elevations in mini-piezometers demonstrate a similar response to seasonal fluctuations as the groundwater monitors. Groundwater elevations were high in late spring, gradually decreasing into the drier summer months. Groundwater elevations were lowest at the end of September in MP2-S and MP2-D, at 507.46 and 507.48 masl, respectively. It should be noted that continuous groundwater elevations are unavailable for MP1-D, MP2-D, and MP4-S between September 20, 2022, and November 25, 2022, as the shallow groundwater levels fell below the depth of the logger. Manual measurements during this time period were used to assess groundwater elevation. Manual measurements collected in March 2023 demonstrate high groundwater elevations, typical of the spring freshet.

4.2.2 Horizontal Groundwater Flow

The interpreted groundwater contours for May 2023, representing a generally high-water table position, are presented in **Figure 10**. Water levels from May 2023 contains the most complete dataset (i.e., wells were not frozen) during spring conditions, which are of particular interest as it typically represents the highest groundwater elevations and will therefore inform the engineering design of residential development. The interpreted groundwater flow direction is generally in a south to southeasterly direction along the west portion of the Site. Along the East portion of the site, the groundwater flow direction south to southwesterly, towards the wetland. Shallow groundwater contours at the Site have generally mimicked surface topography with the horizontal component of groundwater flow travelling in the weathered upper till.

4.2.3 Vertical Groundwater Flow

Vertical hydraulic gradients were calculated between the shallow and deep monitors at the nested monitoring well locations to assess groundwater discharge/recharge conditions across the Site. Vertical hydraulic gradients were also calculated at the mini-piezometer location to assess groundwater-surface water interactions within the natural heritage features located east of the Site. MP4D became damaged (bent) during the monitoring program and therefore vertical gradients were assessed using the shallow MP4S and surface water elevations. The vertical hydraulic gradients are provided in **Table C-3a and C3b, Appendix C**.

Groundwater elevations were higher in the shallow monitor compared to its deeper counterpart at all nested monitoring locations. Measured hydraulic gradients ranged between -0.39 m/m to



0.37 m/m with the maximum and minimum occurring at nested wells MW22-207. Vertical hydraulic gradients were predominantly downward groundwater movement across the Site and were generally minimal in magnitude (0.01 m/m to 0.19 m/m, excluding MW22-207). An upward hydraulic gradient was noted at location MW22-207 (-0.39 m/m) however, this nested location has not historically trended with an upward gradient in any season. It is interpreted that groundwater recharge conditions are present within the silty sand till.

Groundwater elevations at MP1 were also consistently higher in the shallow monitor, indicating groundwater recharge conditions at this location. In the spring of 2023, there is some evidence of groundwater discharge from the logger data at MP1. MP1 is located upgradient of the Melancthon Wetland Complex #1 southeast of the Site. This indicates that this portion of the natural heritage feature is primarily fed by precipitation events and surface water runoff.

At MP4 (which is located in the drainage feature), groundwater recharge conditions were generally observed during the summer months/early fall. It is interpreted that there is more consistent groundwater recharge occurring surrounding this feature, albeit limited by the low permeability surficial soils. The drainage feature flows into the Melancthon Wetland Complex #1. Measured hydraulic gradients within the wetland ranged from -0.11 m/m to 0.10 m/m, as recorded at MP2 and MP3. This indicates that there are minimal groundwater contributions to the wetland and that it is primarily surface water supported. It should be noted that surface water was generally not present at MP2 and MP3 throughout the monitoring period, suggesting limited to no interactions with the groundwater in this wetland.

4.3 Hydraulic Conductivity

In-situ hydraulic conductivity tests were completed at four (4) groundwater monitoring wells at the Site. The results of the hydraulic conductivity tests are provided in **Table 4-1**, and the AQTESOLV analyses are provided in **Appendix D**.

Table 4-1: Hydraulic Conductivity

Monitor	Hydraulic Conductivity (m/s)	Screened Strata
MW22-202 S	2.8 x 10 ⁻⁷	Silty sand, silty sand till
MW22-202 D	1.1 x 10 ⁻⁸	Silty sand till
MW22-203	1.2 x 10 ⁻⁸	Silty sand till
MW22-209	3.6 x 10 ⁻⁷	Silty sand till

The geometric mean hydraulic conductivity for the four (4) tested monitors is 6.0×10^{-8} m/s, with a measured range of 1.1×10^{-8} to 3.6×10^{-8} m/s. The monitoring wells were screened within the silty sand till (with gravel) unit. Although no deeper groundwater monitors were screened in the unweathered glacial till aquitard, experience has shown that the hydraulic conductivity can be as low as 10^{-9} m/s and is typically 10 to 20 times lower than weathered till (Freeze and Cherry, 1979). The results are consistent with those reported by Freeze and Cherry (1979) for similar soils.

4.4 MECP Water Well Record Database

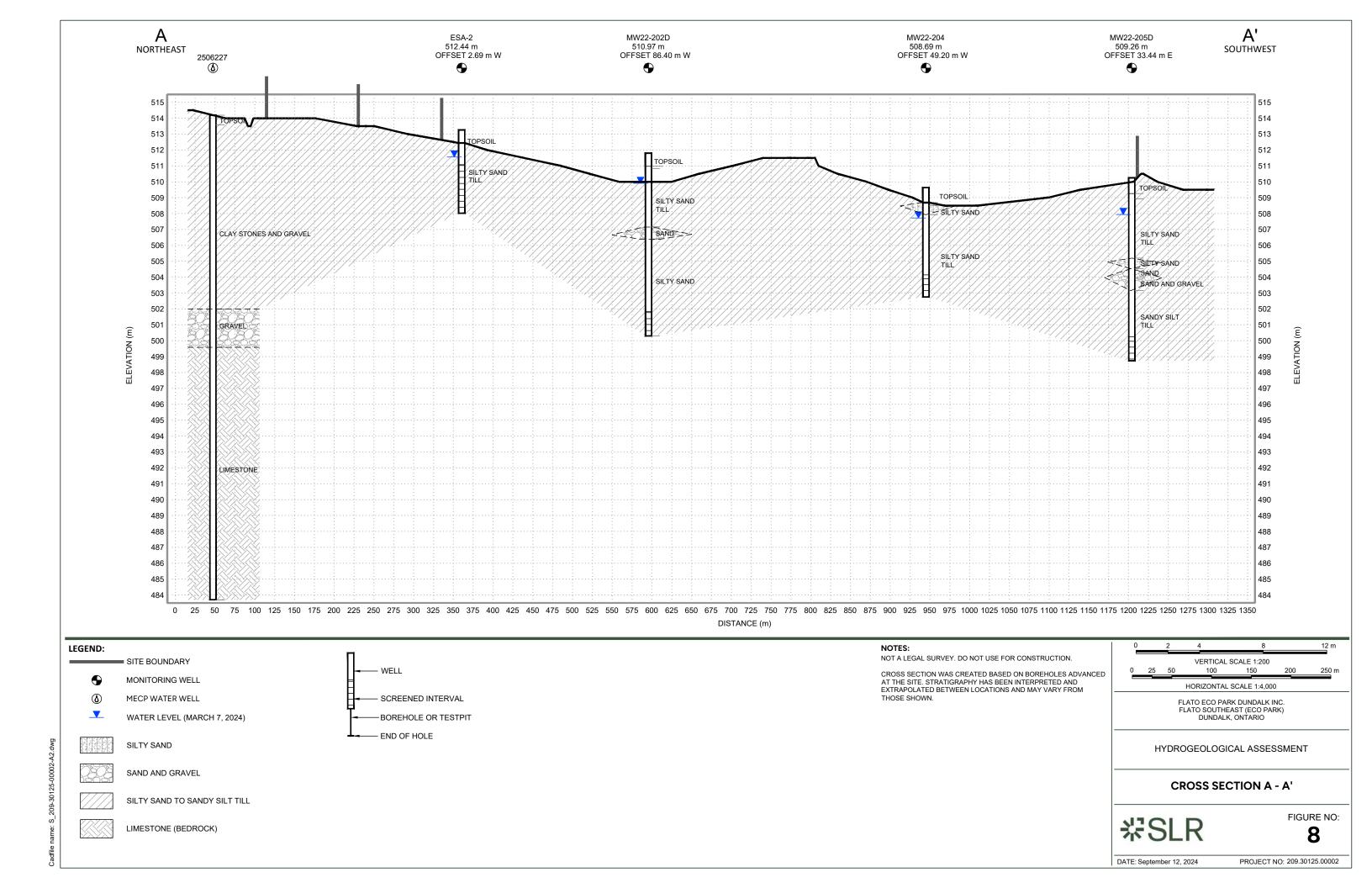
Well records from the MECP WWR database were reviewed to assess the stratigraphy and water use of wells located within a 500 m radius of the Site. The locations of the wells are

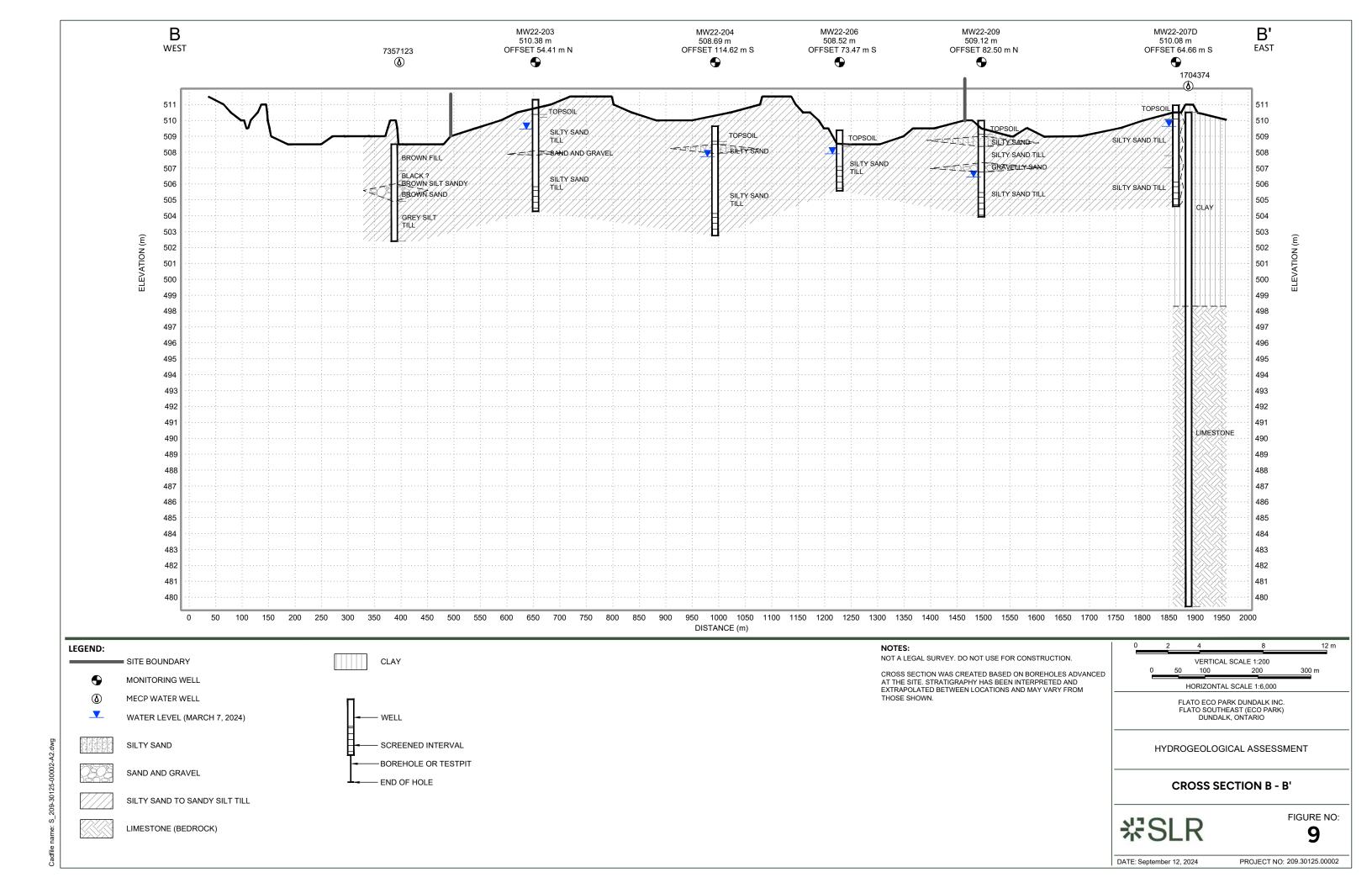


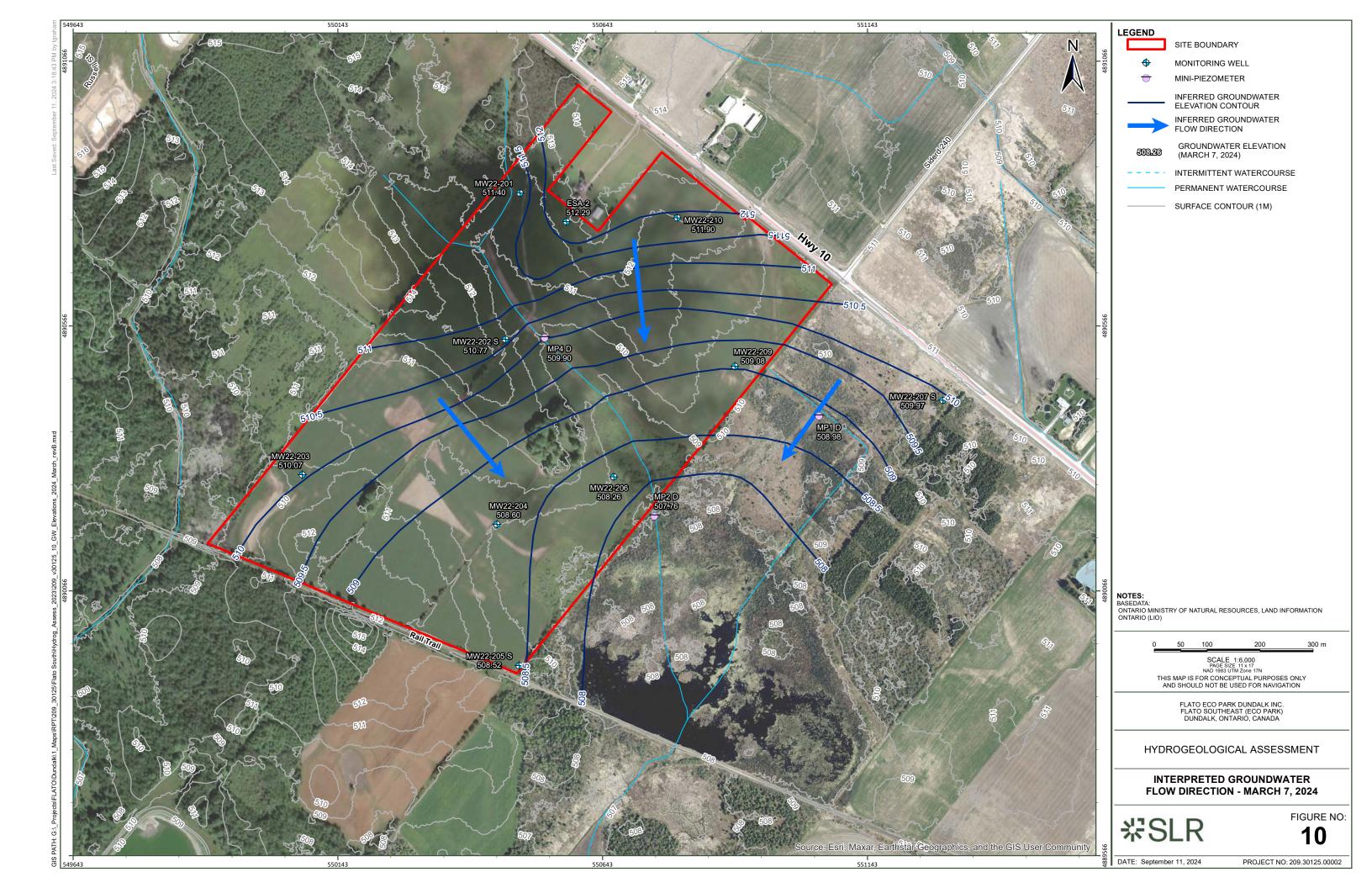
shown in **Figure 11**, and a summary is provided in **Appendix E**. Copies of the well records are provided in **Appendix E**.

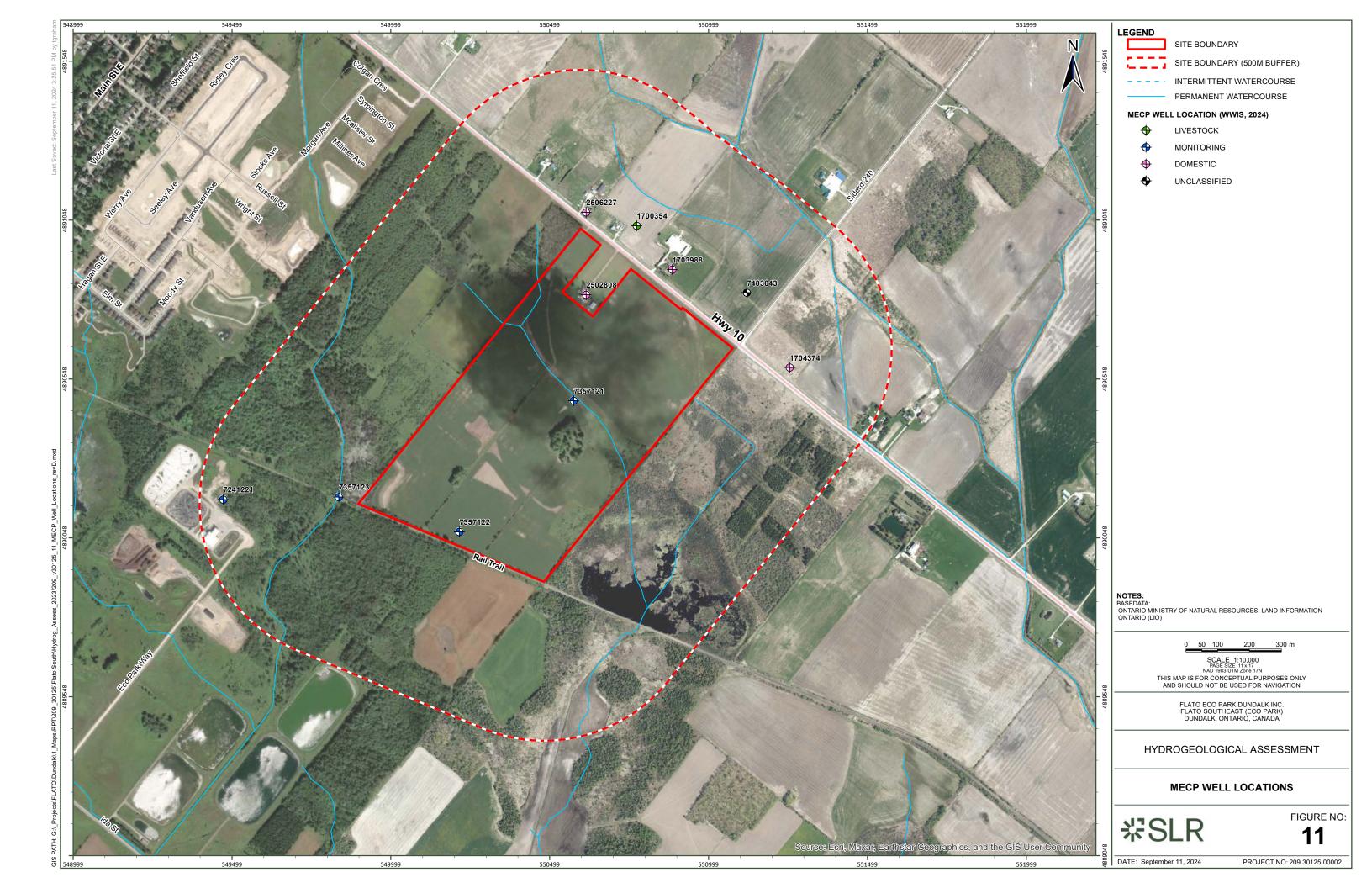
Ten (10) MECP wells were identified within 500 m of the property. Five (5) of those wells were for water supply purposes, four (4) were observation/monitoring wells, and one (1) well without a noted water use. The water supply wells were all completed within the bedrock.











5.0 Impact Assessment for Potential Receptors

5.1 Shallow Groundwater Features

Groundwater elevations across the Site are relatively shallow (generally less than 2 m) in the spring and fluctuate on a seasonal basis. Higher water levels were observed in late winter into spring following precipitation events and spring melt. Water levels decreased into the drier summer months. Water levels generally follow ground surface elevations, where higher groundwater elevations occur at the north edge of site, and lower groundwater elevations occur to the south.

During the spring season, the water table is situated within the surficial silty sand, and sand/gravel pockets that is noted to be discontinuous across the property. Water levels in these monitors drop into the underlying weathered till unit in the drier summer months, and subsequently into the unweathered till. The weathered till unit has an estimated hydraulic conductivity of 2×10^{-7} m/s. Based on a review of the MECP WWR records, the till unit extends to approximately 15 mbgs. Although no deeper groundwater monitors were screened in the unweathered glacial till aquitard, experience has shown that the hydraulic conductivity can be as low as 10^{-9} m/s and is typically 10 to 20 times lower than weathered till (Freeze and Cherry, 1979).

5.2 Aquifers and Potable Water Wells

The Village of Dundalk relies on groundwater supply from wells screened within the dolostone bedrock that extends under the Site. The well capture zones have been documented by the Lake Erie Region Source Protection Committee and extend under the eastern portion of the Site within the bedrock. The upper bedrock is inferred to be of low permeability, and the municipal production zone lies in the middle of the sequence. There are three (3) municipal wells (D3, D4, D5) that service the Village of Dundalk; all wells are located west of the Site, whereby the WHPA-D encroaches along the southwest corner of the Site (**Figure 5**). Rurally there are several surrounding individual residential private wells that tap into the bedrock and have been drilled to depths of approximately 20 to 46 m.

The industrial use for the eastern portion of the Site is currently unspecified. An evaluation of potential impacts and the determination for the need for a RMP or CMP will be evaluated as Site details become available. In addition, the nearby residential water wells are a relatively low draw on the groundwater and given the thickness of the overlying clay aquitard, are not expected to be affected by the proposed development.

Monitoring wells have been installed at the property as part of the Site-specific investigations to document stabilized groundwater conditions. Monitoring is on-going and is planned to continue through construction. When the monitoring wells are determined to be no longer required, or if they are determined to be at risk of damage from Site grading and construction, the wells should be properly decommissioned in accordance with O. Reg. 903. Decommissioning a well which is no longer in use helps ensure the safety of those in the vicinity of the well, prevents surface water infiltration into an aquifer via the well, prevents the vertical movement of water within a well, conserves aquifer yield and hydraulic head, and can potentially remove a physical hazard.

5.3 Natural Environmental Features

There is one drainage feature present within the Site boundary, which flows generally in a south to southeasterly direction into the Melancthon Wetland Complex #1. Groundwater monitoring



completed on Site indicates that there are predominantly downward hydraulic gradient conditions across the site. It has been determined that the drainage feature and Melancthon Wetland Complex #1 are primarily fed by precipitation and surface water-runoff, with limited to no groundwater inputs. As further detailed in the Environmental Impact Study conducted by SLR (2024), removal of the northern portion of the drainage feature during development is not expected to have an impact to the wetland. The southern portion of the drainage feature which will be retained during development and be maintained through mitigation measures as necessary through the established stormwater management plan to be developed by Crozier.

Two (2) stormwater management (SWM) blocks are anticipated to be required to support the proposed development. The SWM ponds will discharge from the east and west to the central environmental protection lands and then to the drainage feature discharging into the wetland south of the proposed development (Crozier Consulting Engineers, 2024). Both SWM blocks will help mitigate the risks from flooding and reduce the chances of sediments and pollutants entering the drainage features and wetland.

5.4 Construction Dewatering

It is expected that temporary excavations for basements will remain dry from a groundwater inflow perspective, due to the low permeability soils and relatively shallow depth of excavation. In the wet season, there may be some temporary groundwater discharge that can be handled by sump and pump techniques. Due to the expected low volumes, it is not expected that Permit to Take Water (PTTW) or Environmental Activity and Sector Registry (EASR) approvals will be required for basement foundations which are anticipated to be fairly shallow.

Additionally, in the event of short-term dewatering, the radius of influence resultant from these temporary excavations is expected to be small, not extending off-Site, therefore, there no impact to the surrounding water supply wells or potable water wells would be anticipated.

Additional evaluations of dewatering requirements, such as the stormwater management block, will be completed during Detailed Design.



6.0 Conclusions

The following presents the conclusions of the Hydrogeological Assessment for the Site.

- The Site is predominantly underlain by surficial silty sand till deposits up to 4.4 m thick. The upper weathered portion of the till unit has an estimated average hydraulic conductivity of 5.7 x 10⁻⁸ m/s.
- Groundwater elevations across the Site fluctuated seasonally between May 2022 and June 2024. Groundwater elevations were highest during the spring monitoring events (506.44 masl to 512.29 masl). Groundwater levels were generally lower during the fall season (506.26 masl to 510.72 masl).
- Groundwater is interpreted to flow primarily in a south to southeasterly direction along the western portion of the Site and is south to southwesterly, towards the wetland, along the eastern portion of the Site.
- Groundwater recharge conditions were observed within the silty sand till, albeit limited by the low permeability of these soils.
- Natural environmental features on-Site were found to be predominantly recharge features supported by precipitation and surface water runoff with little to no groundwater contributions. Groundwater discharge conditions were not observed in the upper reaches of the drainage feature that is planned for removal.
- It is recognized that a small sliver of the southwest corner of the Site is located in a WHPA-D. The property is also located within a SGRA, IPZ, and HVA. Site specific conditions indicated that the subsurface soils across the Site consists of mainly silty sand till. The material was determined to have low hydraulic conductivity and therefore, will act as an aquitard protecting deeper bedrock aquifers.
- Municipal well D4 is located approximately 1.4 km northeast of the Site. In addition, municipal wells D3 and D5 are located approximately 1.5 km and 1.2 km, respectively, east of the Site. No impacts to these wells are anticipated due to the proximal distance of the municipal wells to the Site and low permeability surficial aquitard present at the Site.
- Private wells in close proximity to the Site are primarily completed within the dolostone bedrock and with some in the overburden. The residential water wells are a relatively low draw on the groundwater and given the thickness and low permeability of the overlying till unit, are not expected to be affected by the proposed development.



7.0 Closure

We trust that this report satisfies your requirements at this time.

Regards,

SLR Consulting (Canada) Ltd.

alli Vueni

Allison Vucenovic, M.Sc. Environmental Scientist

1. Cole

Amanda Malatesta, M.Sc., P.Geo. Hydrogeologist

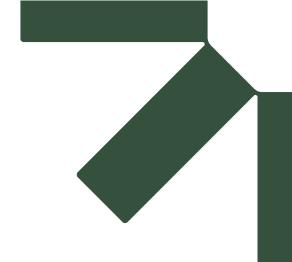
Jason Cole, M.Sc., P.Geo. Principal Hydrogeologist



8.0 References

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- Hvorslev, M.J., 1951. Time Lag and Soil Permeability in Ground-Water Observations, Bull. No. 36, Waterways Exper. Sta. Corps of Engrs, U.S. Army, Vicksburg, Mississippi, pp. 1-50.
- Lake Erie Region Source Protection Committee (2018). Source Water Protection Updated Technical Study for Dundalk Well D5. Revised SPC-18-04-06.
- Lake Erie Region Source Protection Committee. 2021. Grand River Source Protection Area Approved Assessment Report.
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- Ontario Geological Survey (OGS). 2011. Bedrock Geology of Ontario, 1:250 000 scale, Miscellaneous Release Data 126-Revision 1.
- Township of Southgate (2022). Township of Southgate Official Plan. Adopted by Township of Southgate Council: May 4, 2022.





Appendix A Development Plan

Hydrogeological Assessment

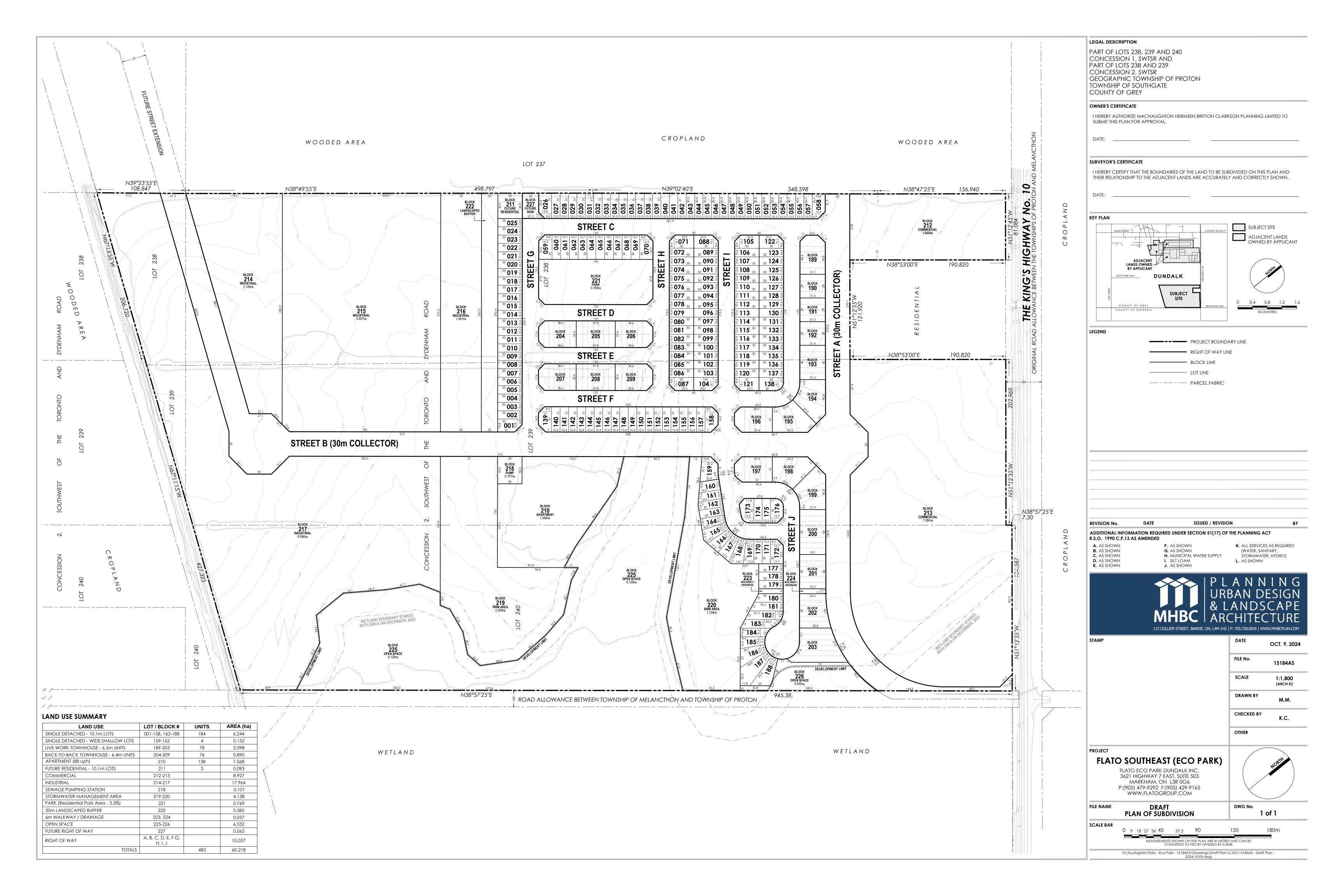
Flato Southeast (Eco Park), Dundalk, Ontario

Flato Eco Park Dundalk Inc.

SLR Project No.: 209.30125.00002

October 11, 2024







Appendix B Borehole Logs

Hydrogeological Assessment

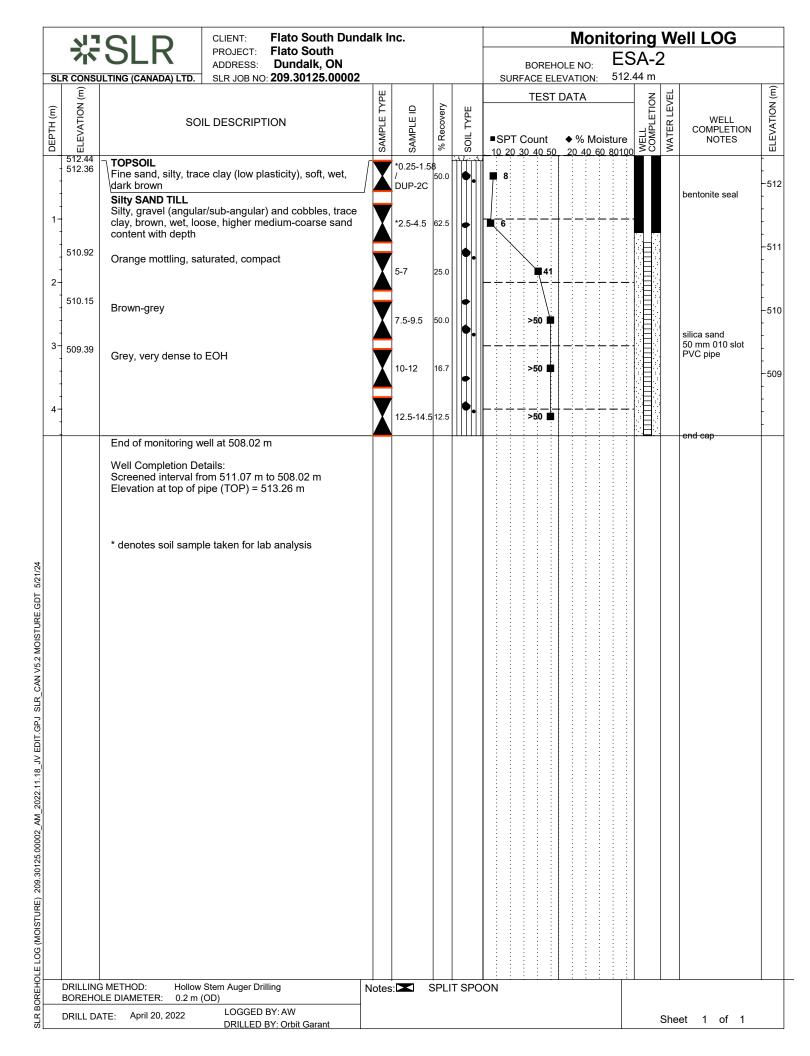
Flato Southeast (Eco Park), Dundalk, Ontario

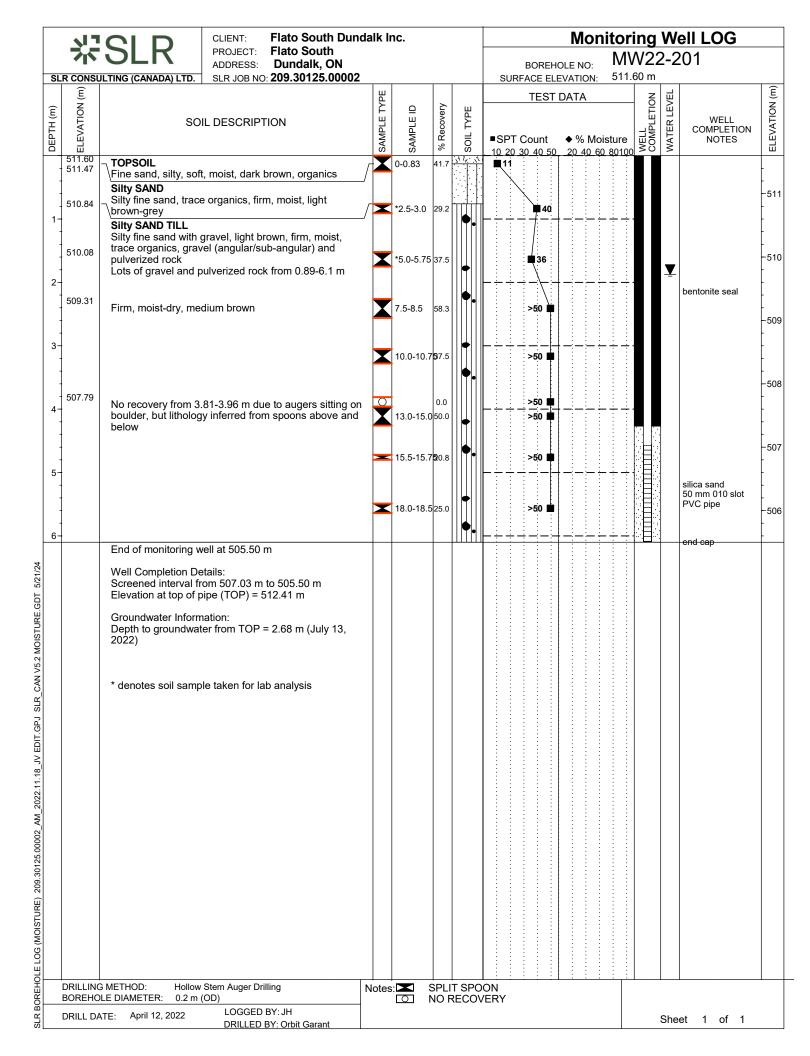
Flato Eco Park Dundalk Inc.

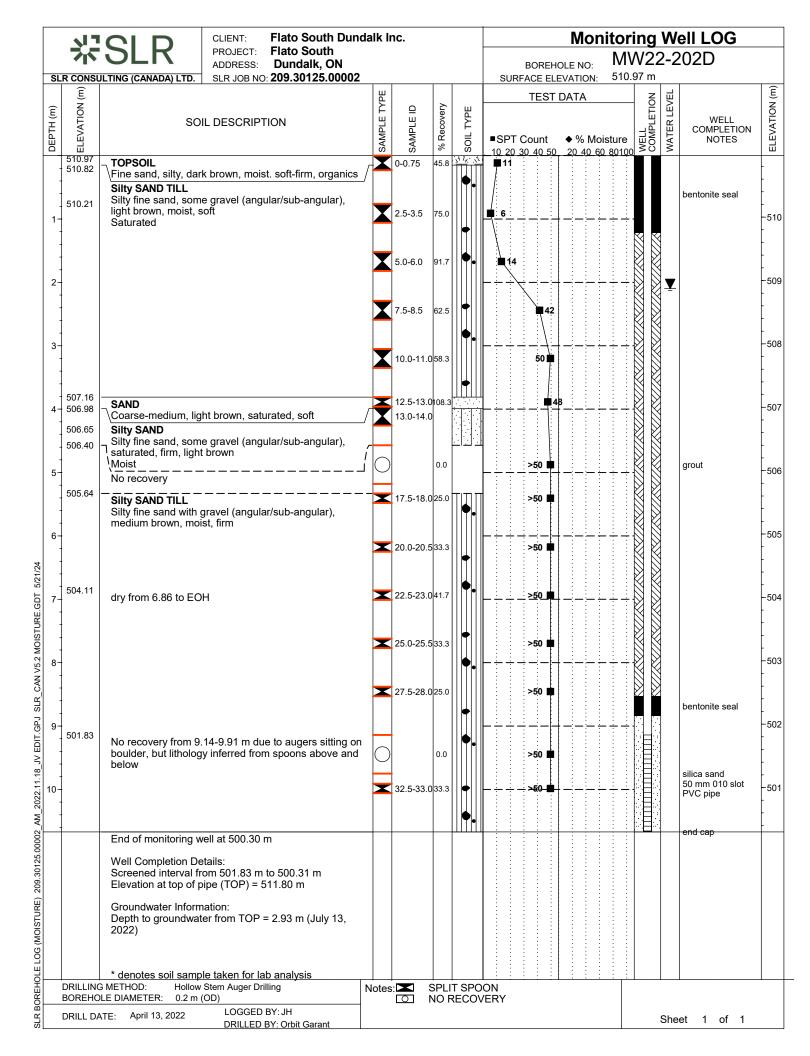
SLR Project No.: 209.30125.00002

October 11, 2024

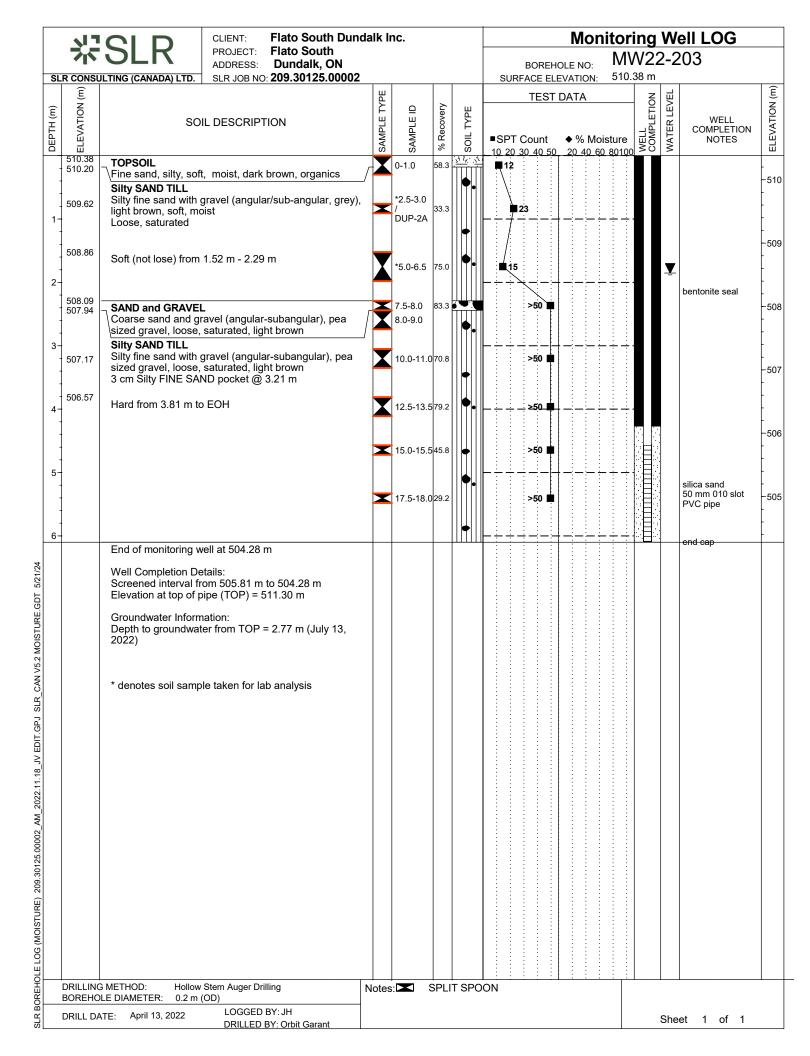


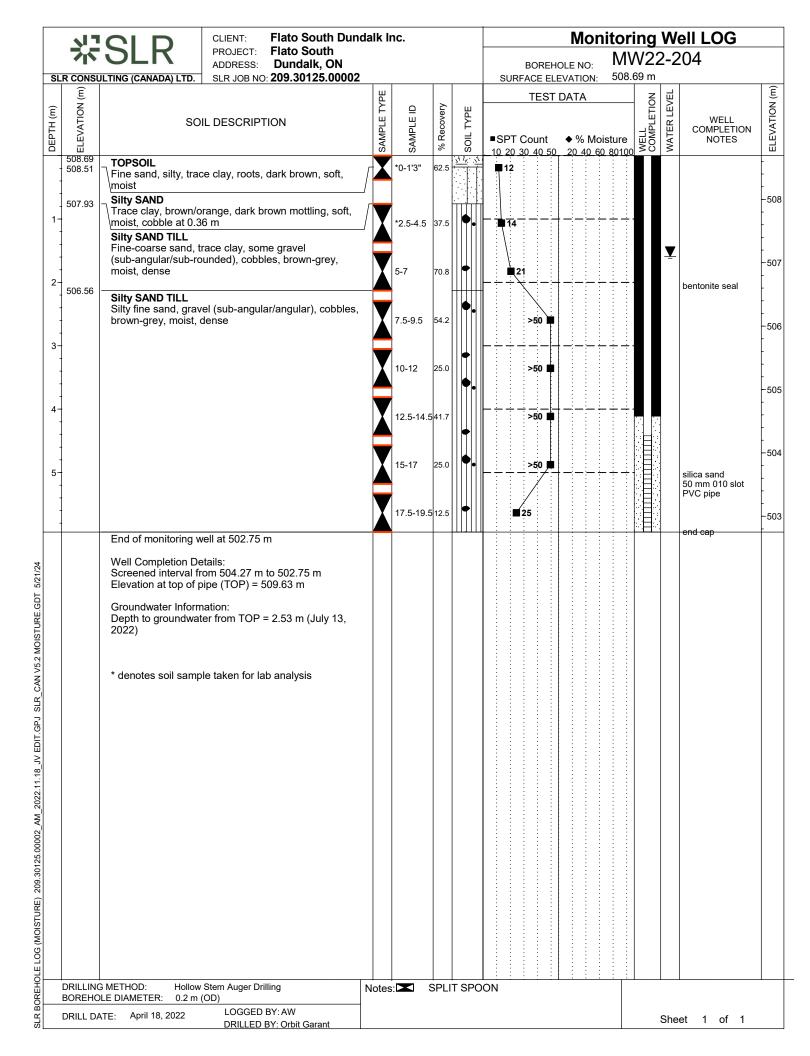


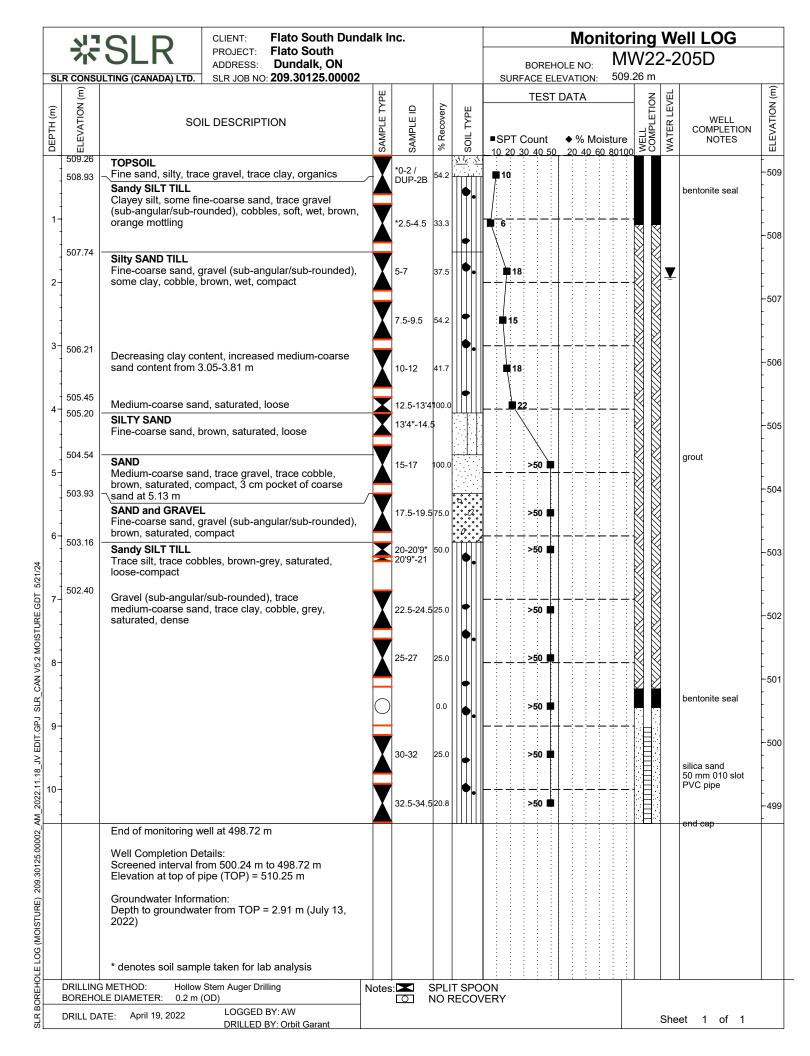




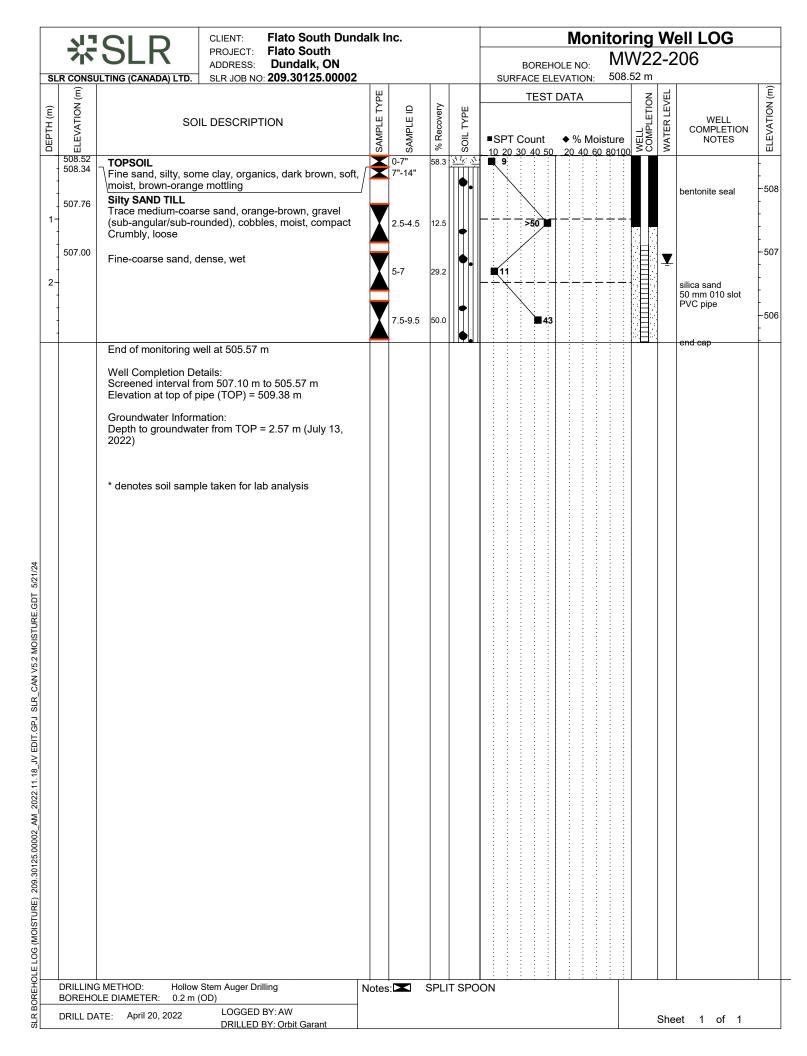
		117	CLIENT: Flato South Dunc	dalk lı	nc.							ell LOG	
			SLR CLIENT: Flato South Dunce Flato South Dunce Flato South Dundalk, ON					BOILE NO.			2-2	.02S	
	SLI		SLR JOB NO: 209.30125.00002	Ш				001117102 222 771110111	510.9 		_		Ê
	Ē	ELEVATION (m)		SAMPLE TYPE	₽	ery	М	TEST DATA	\dashv	WELL COMPLETION	WATER LEVEL		ELEVATION (m)
	DEPIH (m)	ATI	SOIL DESCRIPTION	밀	SAMPLE ID	Recovery	. TYPE			i. P.E	ER I	WELL COMPLETION	VATI
[밁			SAM	SAM	% R	SOIL	■SPT Count ◆ % Moist	ure 0100	WEL	WAT	NOTES	ELE
	-	510.93 510.78	TOPSOIL √Fine sand, silty, dark brown, moist. soft-firm, organics	7			<u> </u>						-
	-		Silty SAND TILL	[/]			••.						
]	510.17	Silty fine sand, some gravel (angular/sub-angular), light brown, moist, soft										
	1-		Saturated					<u> </u>					-510 -
]												-
	1						• •				ϫ		-
	2-							 				bentonite seal	-509
	1												-
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	3-						••						-508
													ŀ
	1												-
	,	507.12	SAND	+									-507
	4-	506.94	Coarse-medium, light brown, saturated, soft	Л									-
		506.61 506.36	Silty SAND Silty fine sand, some gravel (angular/sub-angular),										-
	-	000.00	saturated, firm, light brown Moist	<i>[</i>]									500
	5-		No recovery	'								silica sand 50 mm 010 slot	-506 -
	-	505.60	Silty SAND TILL	+			ТШП					PVC pipe	-
	1		Silty fine sand with gravel (angular/sub-angular), medium brown, moist, firm				▼•					and can	-
	6-		End of monitoring well at 504.83 m									end cap	-505
/24			Well Completion Details:										
5/21/24			Screened interval from 506.59 m to 505.06 m										
GDT.			Elevation at top of pipe (TOP) = 511.86 m										
I GRE			Groundwater Information: Depth to groundwater from TOP = 2.63 m (July 13,										
NOIS			2022)										
CAN V5.2 MOISTURE.GDT													
			* denotes soil sample taken for lab analysis										
SLR			MW22-202S was straight drilled directly adjacent to										
GP.			MW-202D										
2022.11.18_JV EDIT.GPJ													
8													
2.11.1													
202													
AM													
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IRE)													
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SLR BOREHOLE LOG (MOISTURE)			METHOD: Hollow Stem Auger Drilling	Notes	 ::				\top				1
BOR			LE DIAMETER: 0.2 m (OD)										
SLR	-	DRILL DA	TE: April 14, 2022 LOGGED BY: JH DRILLED BY: Orbit Garant								She	et 1 of 1	

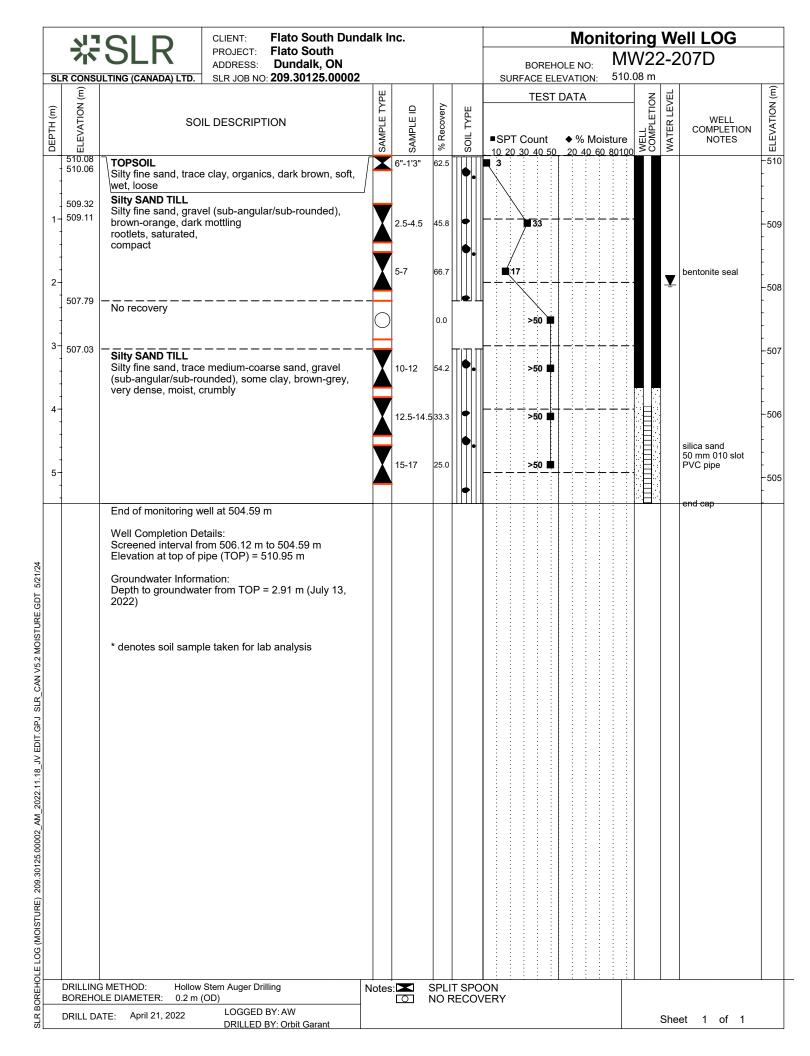




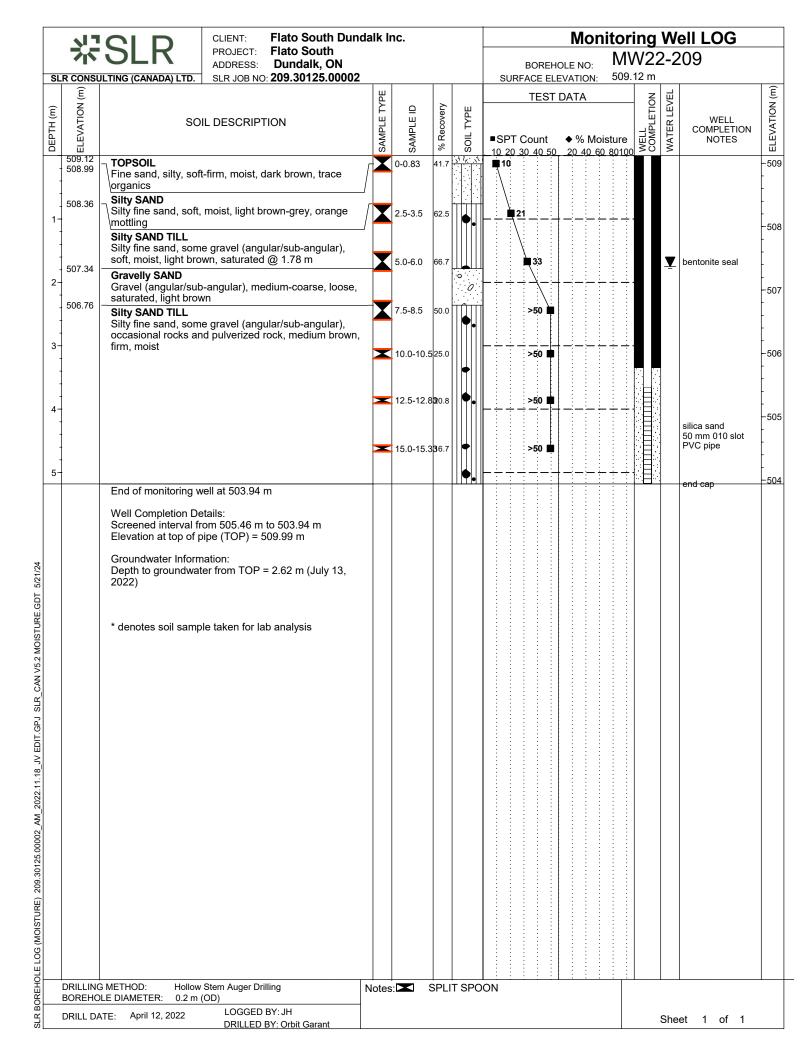


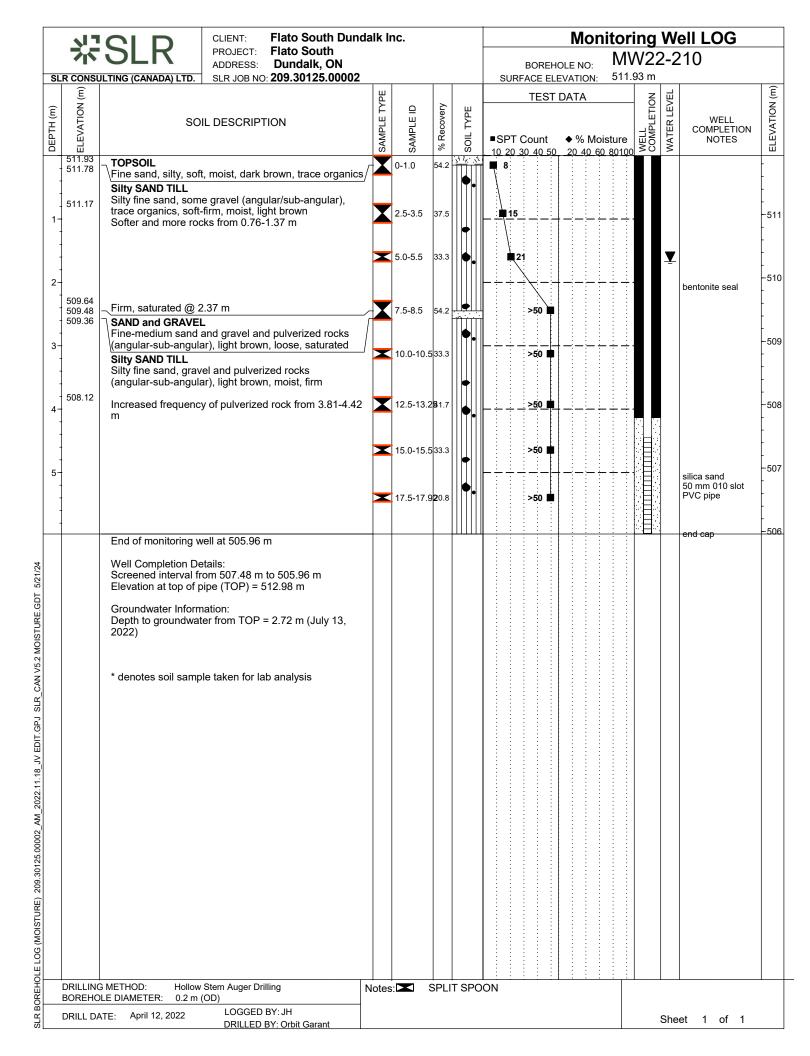
	兴	CLIENT: Flato South Dund	ain ii	16.			N			<u>ell LOG</u> 205S	_
SL		ADDRESS: Dundalk, ON SLR JOB NO: 209.30125.00002					DONLITOLL NO.	/IVV∠ 09.36 m		2000	
	(m) NOI		TYPE	<u> </u>	'ery	TYPE	TEST DATA	NO.	LEVEL		
	ELEVATION (m)	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE ID	% Recovery	SOIL TY	■SPT Count ◆ % Moistur		WATER LEVEL	WELL COMPLETION NOTES	
	509.36	TOPSOIL Fine sand, silty, trace gravel, trace clay, organics				7/1/N 7/1	- 10 20 30 40 00 20 40 00 00 - : : : : : : : : : : : : : : : : : : :	: :			Ŧ
1-	509.03	Sandy SILT TILL Clayey silt, some fine-coarse sand, trace gravel (sub-angular/sub-rounded), cobbles, soft, wet, brown, orange mottling						· · ·			
2-	507.84	Silty SAND TILL Fine-coarse sand, gravel (sub-angular/sub-rounded), some clay, cobble, brown, wet, compact				•.			Ā	bentonite seal	-
3-	506.31	Decreasing clay content, increased medium-coarse sand content from 3.05-3.81 m				• .					-
-	505.55	Medium-coarse sand, saturated, loose				•		:			ŀ
4-	505.30	SILTY SAND Fine-coarse sand, brown, saturated, loose									
1	504.64	SAND	+							silica sand	}
5-	504.03	Medium-coarse sand, trace gravel, trace cobble, brown, saturated, compact, 3 cm pocket of coarse sand at 5.13 m				*****				50 mm 010 slot PVC pipe	-
1		SAND and GRAVEL Fine-coarse sand, gravel (sub-angular/sub-rounded),								and ac-	F
		\brown, saturated, compact End of monitoring well at 503.47 m								end cap	1
		Well Completion Details:									
		Screened interval from 504.99 m to 503.47 m Elevation at top of pipe (TOP) = 510.29 m									
		Groundwater Information: Depth to groundwater from TOP = 2.87 m (July 13, 2022)									
		* denotes soil sample taken for lab analysis									
		MW22-205S was straight drilled directly adjacent to									
		MW22-205D									
	DD		\perp								
		G METHOD: Hollow Stem Auger Drilling DLE DIAMETER: 0.2 m (OD)	Notes	:							
_	DRILL D	ATE: April 19, 2022 LOGGED BY: AW DRILLED BY: Orbit Garant							She	et 1 of 1	

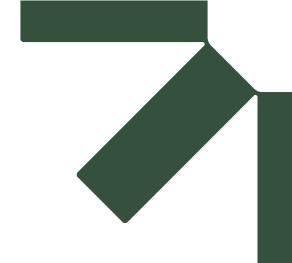




	<u> </u>	CI D	CLIENT: Flato South Dunda	alk Ir	1C.			Monitoring Well LOG				ell LOG	
-		SLR JILTING (CANADA) LTD.	PROJECT: Flato South ADDRESS: Dundalk, ON SLR JOB NO: 209.30125.00002					BOREHOLE NO SURFACE ELEVATIO	,. 	W2 :		207S	
	_	DETING (CANADA) ETD.	3LN JOB NO. 203.30 123.00002	Щ				TEST DATA	N. 010	_	1		Œ
DEPTH (m)	ELEVATION (m)	so	IL DESCRIPTION	SAMPLE TYPE	SAMPLE ID	% Recovery	SOIL TYPE	■SPT Count ◆%	Moisture	WELL COMPLETION	WATER LEVEL	WELL COMPLETION NOTES	ELEVATION (m)
	510.10 510.08	TOPSOIL		- 0,	0)	+		10 20 30 40 50 20 40	0 8010	0 20			-510
	509.34	wet, loose Silty SAND TILL	e clay, organics, dark brown, soft,				•					bentonite seal	-
1	509.13	brown-orange, dark rootlets, saturated, compact	er (sub-angular/sub-rounded), mottling				+ •.						-509 -
2	507.81	No recovery		-							T	silica sand 50 mm 010 slot PVC pipe	- -508 - -
	.												-
3	507.05	Silty SAND TILL Silty fine sand, trace (sub-angular/sub-ro very dense, moist, o	e medium-coarse sand, gravel nunded), some clay, brown-grey, crumbly	1			—					bentonite seal	-507 - - -
		End of monitoring w	vell at 506.14 m										
			etails: om 508.58 m to 507.05 m pipe (TOP) = 510.93 m										
		Groundwater Inform Depth to groundwat 2022)	nation: er from TOP = 2.82 m (July 13,										
/24		* denotes soil samp	le taken for lab analysis										
RE.GDT 5/21/24													
BOREHOLE LOG (MOISTURE) 209.30125.00002_AM_2022.11.18_JV EDIT.GPJ SLR_CAN V5.2 MOISTU													
V EDIT.GPJ													
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MOISTURE)													
LOG (
HOLE	DRILLING	METHOD: Hollow	Stem Auger Drilling N	Notes	:				<u> </u>				<u></u>
BORE	BOREHO	DLE DIAMETER: 0.2 m		.0103	•								
SLR	DRILL DA	ATE: April 22, 2022	DRILLED BY: Orbit Garant								She	et 1 of 1	







Appendix C Groundwater Data

Hydrogeological Assessment

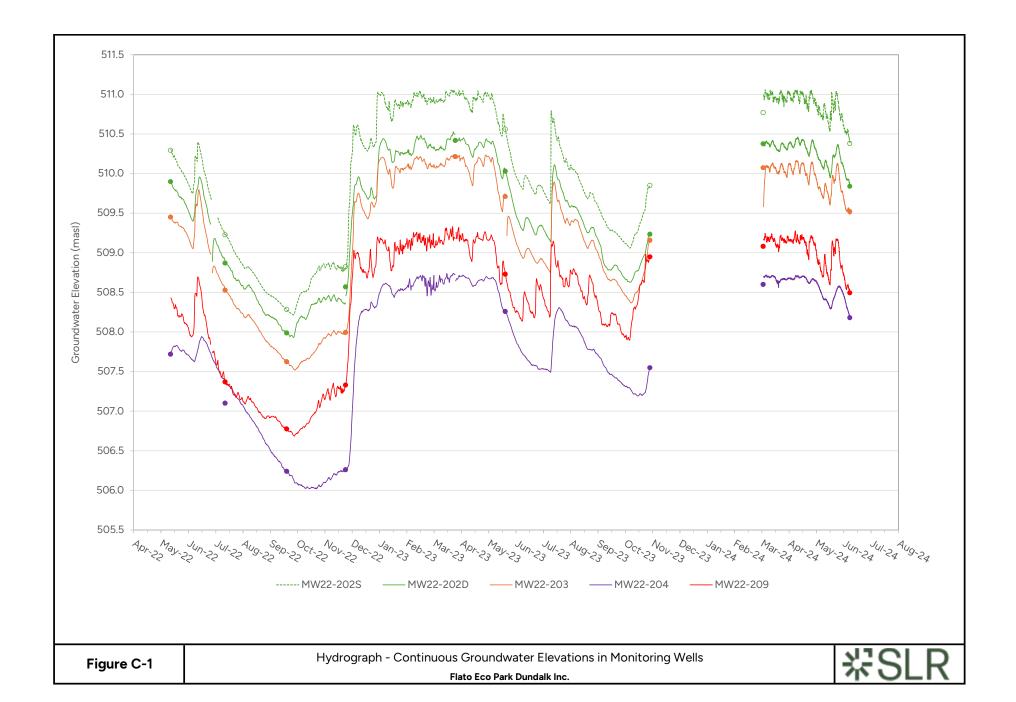
Flato Southeast (Eco Park), Dundalk, Ontario

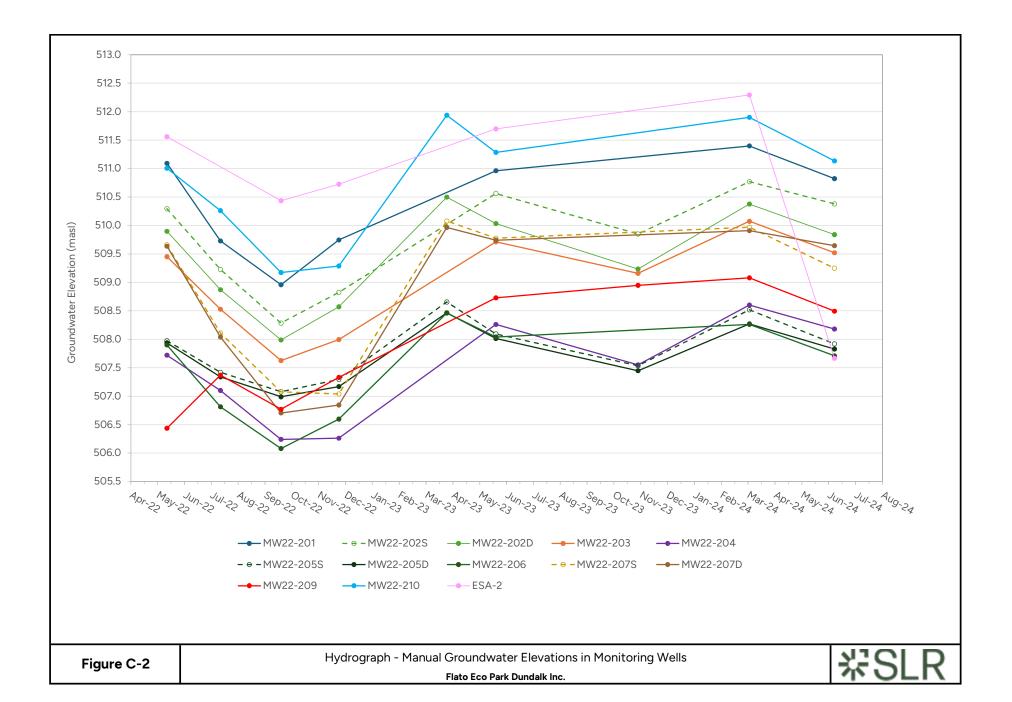
Flato Eco Park Dundalk Inc.

SLR Project No.: 209.30125.00002

October 11, 2024







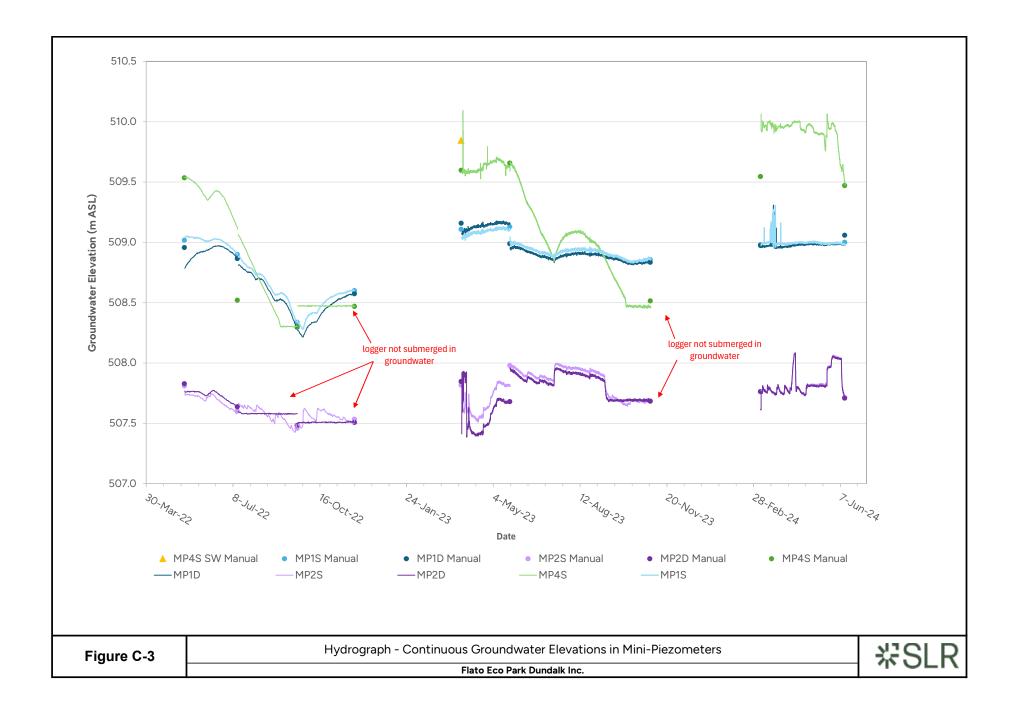


Table C-1: Monitoring Well Groundwater Elevations

Monitor ID	Units	13-May-22	13-Jul-22	20-Sep-22	25-Nov-22	28-Mar-23	23-May-23	1-Nov-23	7-Mar-24	12-Jun-24
ESA-2	mbgs	0.88	-	2.01	1.72	-	0.74	-	0.15	4.78
LJA-2	masl	511.56	-	510.44	510.72	-	511.70	-	512.29	507.66
MW22-201	mbgs	0.51	1.87	2.64	1.86	-	0.64	-	0.20	0.78
1010022-201	masl	511.09	509.73	508.96	509.75	-	510.96	-	511.40	510.82
MW22-202 S	mbgs	0.63	1.70	2.64	2.11	-	0.37	1.08	0.16	0.55
1010022-2023	masl	510.30	509.23	508.29	508.82	-	510.56	509.85	510.77	510.38
MW22-202 D	mbgs	1.07	2.10	2.98	2.40	0.47	0.94	1.74	0.59	1.13
IVIVV22-202 D	masl	509.90	508.87	507.99	508.57	510.50	510.04	509.24	510.38	509.84
MW22-203	mbgs	0.93	1.85	2.75	2.38	0.17	0.67	1.22	0.31	0.86
1010022-203	masl	509.45	508.53	507.63	508.00	510.21	509.71	509.16	510.07	509.52
MW22-204	mbgs	0.97	1.59	2.45	2.43	-	0.43	1.14	0.09	0.51
1010022-204	masl	507.72	507.10	506.24	506.26	-	508.26	507.55	508.60	508.18
MW22-205 S	mbgs	1.39	1.94	2.28	2.06	0.70	1.27	1.82	0.84	1.44
1010022-2053	masl	507.97	507.42	507.08	507.30	508.66	508.09	507.54	508.52	507.92
MW22 205 D	mbgs	1.32	1.92	2.27	2.09	0.80	1.25	1.81	0.99	1.43
MW22-205 D	masl	507.94	507.34	506.99	507.17	508.46	508.01	507.45	508.27	507.83
MM/22 20C	mbgs	0.62	1.70	2.44	1.92	0.06	0.48	-	0.26	0.81
MW22-206	masl	507.90	506.82	506.08	506.60	508.46	508.04	-	508.26	507.71
MW22-207 S	mbgs	0.44	1.99	3.02	3.06	0.02	0.32	-	0.13	0.85
1010022-2073	masl	509.66	508.11	507.08	507.04	510.08	509.78	-	509.97	509.25
NAVA/22 207 D	mbgs	0.44	2.04	3.38	3.24	0.11	0.34	-	0.17	0.43
MW22-207 D	masl	509.64	508.04	506.71	506.84	509.97	509.74	-	509.91	509.65
MM/22 200	mbgs	2.68	1.75	2.35	1.79	-	0.39	0.17	0.04	0.62
MW22-209	masl	506.44	507.37	506.77	507.33	-	508.73	508.95	509.08	508.50
NAVA22 240	mbgs	0.92	1.67	2.76	2.64	-0.01	0.64	-	0.03	0.79
MW22-210	masl	511.01	510.26	509.17	509.29	511.94	511.29	-	511.90	511.14

mbgs metres below ground surface masl metres below sea level

Table C-2: Mini-Piezometer Groundwater Elevations

Monitor ID	Units	13-May-22	13-Jul-22	20-Sep-22	25-Nov-23	28-Mar-23	23-May-23	1-Nov-23	7-Mar-24	12-Jun-24
MP1S	mbgs	-0.17	-0.05	0.51	0.25	-0.26	-0.28	-0.01	-0.13	-0.15
IVIF13	masl	509.02	508.90	508.34	508.60	509.11	509.13	508.86	508.98	509.00
MP1D	mbgs	-0.12	-0.03	0.54	0.26	-0.32	-0.15	0.005	-0.14	-0.22
MINID	masl	508.96	508.87	508.30	508.58	509.16	508.99	508.84	508.98	509.06
MDDC	mbgs	-0.29	-0.08	0.06	-0.02	-0.30	-0.46	-0.21	-0.24	-0.20
MP2S	masl	507.81	507.60	507.46	507.54	507.82	507.98	507.73	507.76	507.72
MP2D	mbgs	-0.31	-0.12	0.04	0.01	-0.33	-0.46	-0.17	-0.24	5.39
IVIPZD	masl	507.83	507.64	507.48	507.51	507.85	507.98	507.69	507.76	502.13
MD2C	mbgs	-	-0.33	-0.19	-	-	-0.62	-	-	-
MP3S	masl	-	507.72	507.58	-	-	508.01	-	-	-
MP3D	mbgs	-	-0.35	-0.25	-	-	-0.57	-	-	-
IVIP3D	masl	-	507.74	507.64	-	-	507.96	-	-	-
MDAC	mbgs	0.06	1.08	1.30	1.13	0.002	-0.06	1.09	0.05	0.13
MP4S	masl	509.54	508.52	508.30	508.47	509.60	509.66	508.52	509.55	509.47
MDAD	mbgs	-0.09	1.11	1.62	1.51	-0.42	-0.17	0.76	-0.28	0.21
MP4D	masl	509.71	508.51	508.01	508.11	510.04	509.79	508.86	509.90	509.42

mbgs metres below ground surface masl metres below sea level

MP4D piezometer was noted damaged (bent) from March 2023 onward

Table C-3a: Vertical Hydraulic Gradients - Monitoring Wells

Monitor ID	13-May-22	13-Jul-22	20-Sep-22	25-Nov-22	28-Mar-23	23-May-23	7-Mar-24	12-Jun-24			
			MW22-202								
Shallow groundwater elevations (masl)	510.30	509.23	508.29	508.82	Frozen	510.56	510.77	510.38			
Deep groundwater elevations (masl)	509.90	508.87	507.99	508.57	510.50	510.04	510.38	509.84			
Hydraulic gradient (m/m)	0.12	0.11	0.09	0.08	-	0.16	0.12	0.16			
MW22-205											
Shallow groundwater elevations (masl)	507.97	507.42	507.08	507.30	508.66	508.09	508.52	507.92			
Deep groundwater elevations (masl)	507.94	507.34	506.99	507.17	508.46	508.01	508.27	507.83			
Hydraulic gradient (m/m)	0.01	0.02	0.03	0.04	0.06	0.02	0.08	0.03			
			MW22-207								
Shallow groundwater elevations (masl)	509.66	508.11	507.08	507.04	510.08	509.78	509.97	509.25			
Deep groundwater elevations (masl)	509.64	508.04	506.71	506.84	509.97	509.74	509.91	509.65			
Hydraulic gradient (m/m)	0.02	0.07	0.37	0.19	0.11	0.04	0.06	-0.39			

masl denotes metres above sea level

Positive value denotes downward hydraulic gradients (i.e., groundwater recharge conditions)
Negative value denotes upward hydraulic gradients (i.e., groundwater discharge conditions)
N.R. denotes not representative as water levels did not fully recover following installation

Table C-3b: Vertical Hydraulic Gradients - Mini-Piezometers

Monitor ID	13-May-22	13-Jul-22	20-Sep-22	25-Nov-22	28-Mar-23	23-May-23	7-Mar-24	12-Jun-24
			MP1					
Shallow groundwater elevations (masl)	509.02	508.90	508.34	508.60	509.11	509.13	508.98	509.00
Deep groundwater elevations (masl)	508.96	508.87	508.30	508.58	509.16	508.99	508.98	509.06
Hydraulic gradient (m/m)	0.10	0.06	0.07	0.04	-0.08	0.23	0.01	-0.10
			MP2					
Shallow groundwater elevations (masl)	507.81	507.60	507.46	507.54	507.82	507.98	507.76	507.72
Deep groundwater elevations (masl)	507.83	507.64	507.48	507.51	507.85	507.98	507.76	507.71
Hydraulic gradient (m/m)	-0.03	-0.05	-0.03	0.04	-0.04	0.00	0.00	0.01
			MP3					
Shallow groundwater elevations (masl)	-	507.72	507.58	-	-	508.01	-	
Deep groundwater elevations (masl)	-	507.74	507.64	-	-	507.96	-	
Hydraulic gradient (m/m)	-	-0.05	-0.11	-	-	0.10	-	
	,		MP4					
Shallow groundwater elevations (masl)	-	-	-	-	509.85	-	-	-
Deep groundwater elevations (masl)	509.54	508.52	508.30	508.52	509.60	509.66	509.55	509.47
Hydraulic gradients (m/m) ¹					0.21			

masl denotes metres above sea level

Positive value denotes downward hydraulic gradients (i.e., groundwater recharge conditions)

Negative value denotes upward hydraulic gradients (i.e., groundwater discharge conditions)

¹ - Hydraulic gradient calculated between shallow MP and surface water when present



Appendix D Hydraulic Conductivity Analyses

Hydrogeological Assessment

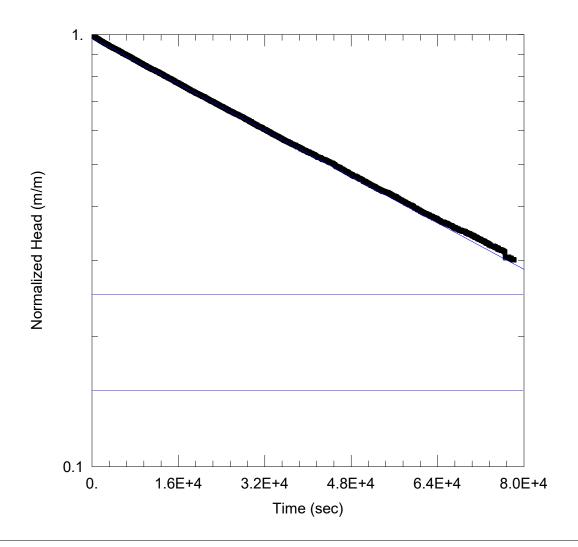
Flato Southeast (Eco Park), Dundalk, Ontario

Flato Eco Park Dundalk Inc.

SLR Project No.: 209.30125.00002

October 11, 2024





Data Set: N:\...\MW22-202D_JH_JV hvorslev.aqt

Date: 09/09/24 Time: 17:14:46

PROJECT INFORMATION

Company: <u>SLR</u> Client: Flato

Project: 209.30125.00002 Location: Dundalk South Test Well: MW22-202D

AQUIFER DATA

Saturated Thickness: 8.924 m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (MW22-202D)

Initial Displacement: 1.541 m

Static Water Column Height: 8.924 m

Total Well Penetration Depth: 8.924 m

Screen Length: 1.524 m Well Radius: 0.1016 m

Casing Radius: 0.0254 m

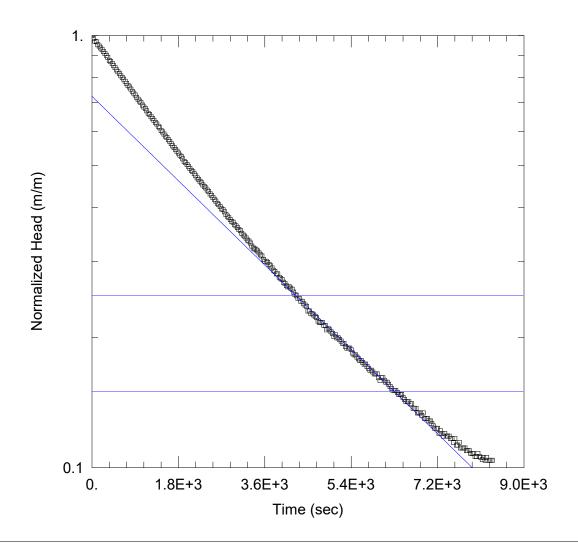
SOLUTION

Aquifer Model: Confined

Solution Method: Hvorslev

K = 1.111E-8 m/sec

y0 = 1.515 m



Data Set: N:\...\MW22-202S_JH_JV hvorslev.aqt

Date: 09/12/24 Time: 11:06:19

PROJECT INFORMATION

Company: <u>SLR</u> Client: Flato

Project: 209.30125.00002 Location: Dundalk South Test Well: MW22-202S

AQUIFER DATA

Saturated Thickness: 1.52 m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (New Well)

Initial Displacement: 1.055 m

Static Water Column Height: 4.512 m

Total Well Penetration Depth: 4.512 m

Screen Length: 1.524 m Well Radius: 0.1016 m

Casing Radius: 0.0254 m

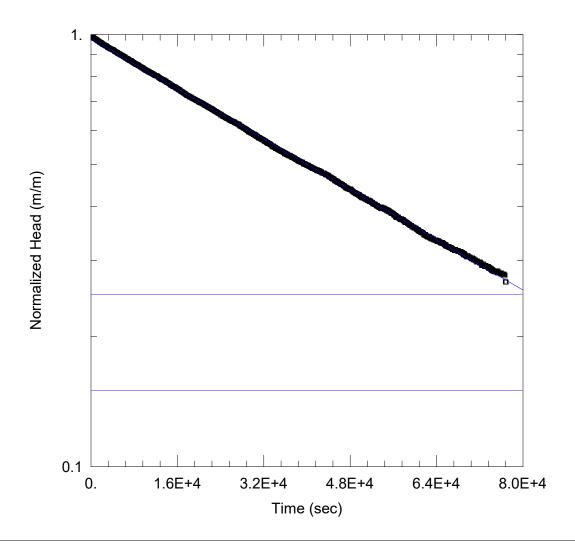
SOLUTION

Aquifer Model: Confined

Solution Method: Hvorslev

K = 2.809E-7 m/sec

y0 = 0.7633 m



Data Set: N:\...\MW22-203 JH JV hvorslev.aqt

Date: 09/09/24 Time: 17:16:44

PROJECT INFORMATION

Company: SLR Client: Flato

Project: 209.30125.00002 Location: Dundalk South Test Well: MW22-203

AQUIFER DATA

Saturated Thickness: 4.685 m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (MW22-203)

Initial Displacement: 1.068 m

Total Well Penetration Depth: 4.685 m

Casing Radius: 0.0254 m

Static Water Column Height: 4.685 m

Screen Length: 1.524 m Well Radius: 0.1016 m

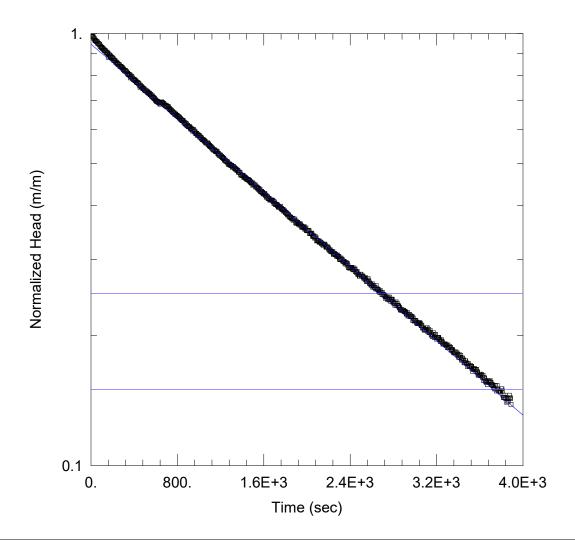
SOLUTION

Aquifer Model: Confined

Solution Method: Hvorslev

K = 1.207E-8 m/sec

y0 = 1.046 m



Data Set: N:\...\MW22-209_JH_JV hvorslev.aqt

Date: 09/09/24 Time: 17:17:17

PROJECT INFORMATION

Company: <u>SLR</u> Client: Flato

Project: 209.30125.00002 Location: Dundalk South Test Well: MW22-209

AQUIFER DATA

Saturated Thickness: 3.854 m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (MW22-209)

Initial Displacement: 1.065 m

Total Well Penetration Depth: 3.854 m

Static Water Column Height: 3.854 m

Casing Radius: 0.0254 m

Screen Length: 1.524 m Well Radius: 0.1016 m

SOLUTION

Aquifer Model: Confined

Solution Method: Hvorslev

K = 3.56E-7 m/sec

y0 = 1.007 m



Appendix E MECP Water Well Records

Hydrogeological Assessment

Flato Southeast (Eco Park), Dundalk, Ontario

Flato Eco Park Dundalk Inc.

SLR Project No.: 209.30125.00002

October 11, 2024



Table E-1: MECP Water Well Record Summary

WELL ID	TAG	DATE COMPLETED	DEPTH (M)	BOTTOM LITHOLOGY	STRATIGRAPHY LAYER	FORMATION	FORMATION END DEPTH (M)	STATUS	WATER USE	DEPTH WATER FOUND (M)	STATIC LEVEL (M)	PUMPING RATE (LPS)	
					1	TOPSOIL	0.61						
.====		= /2 / + 2 = 4			2	HARDPAN	16.15						
1700354		7/2/1954	29	SHALE	3	ROCK	28.04	Water Supply	Livestock	28.0	2.7	0.758	
					4	SHALE	28.96						
		- 4 4			1	CLAY	15.54						
1703988		8/17/1989	45.7	LIMESTONE	2	LIMESTONE	45.72	Water Supply	Domestic	45.7	6.7	0.531	
					1	CLAY	5.79						
1704374		10/9/1991	31.1	LIMESTONE	2	CLAY	12.19	Water Supply	Domestic	25.9	3.7	0.91	
		, ,			3	LIMESTONE	31.09	1					
					1	TOPSOIL	0.30						
2502808		4/10/1969	20.1	ROCK	2	MEDIUM SAND	4.57	Water Supply	Domestic	18.3		1.137	
2502808		4/10/1969	20.1	RUCK	3	GRAVEL	12.19			18.3	3	1.137	
					4	ROCK	20.12						
					1	TOPSOIL	0.61						
2506227		5/19/1976	30.5	LIMESTONE	2	CLAY	12.19	Water Supply	D	20.5	2.4	0.758	
2506227		5/19/19/6	30.5	LIMESTONE	3	GRAVEL	14.63	water Supply	Domestic	Domestic	30.5	2.4	0.758
					4	LIMESTONE	30.48						
7241221	A176434	4/29/2015	4.5	SAND	1	SAND	4.50	Observation Wells	Monitoring				
					1	TOPSOIL	0.60						
7357121	A282537	1/16/2020	5.1	SILT	2	SAND	3.60	Observation Wells	Monitoring				
					3	SILT	5.10						
					1	TOPSOIL	0.60						
7357122	A282216	1/14/2020	4.5	SILT	2	SAND	3.60	Observation Wells	Monitoring				
					3	SILT	4.50						
					1		1.67						
					2		2.30						
7357123	A282615	1/14/2020	6.1	SILT	3	SILT	2.60	Observation Wells	Monitoring				
					4	SAND	3.60						
					5	SILT	6.10	<u> </u>					
7403043		4/27/2021											

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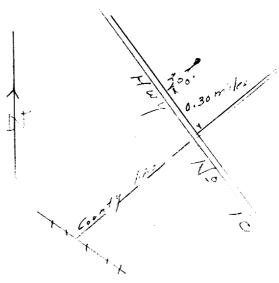
GEOLOGICAL BRANCH
DEPARTMENT of LINES

Nº	354
	/ \

The Water-well Drillers Act, 19
Department of Mines

Water-Well Record

County or Territorial District	PRE-1	$\mathbf{r}_{\alpha \mathbf{w} \mathbf{n}}$	shin	Vinage, Town of C	MELANC	HION
County or Territorial District	V(.3	10411	37	illage, Town or City	7)	
(day)	(month)	(year)				
Pipe and Casing	Record			P	umping Test	
Casing diameter(s) # " Length(s) 53 Type of screen Length of screen			Stat Pun Pun Dur	nping rate	Yater Record	in to
Well Log	1			Depth(s)		Kind of water
Overburden and Bedrock Record	From ft.	To ft.		at which water(s) found	No. of feet water rises	(fresh, salty, or sulphur)
TOP SOIL. HARD PAN. BLUE ROCK SHALE 1.	0 (2 53 (92 (2 (53) 92 (95)	/	92'- 95'	86'	feel
For what purpose(s) is the water STOCK - DOMEST Is water clear or cloudy?Ch Is well on upland, in valley, or or UPLAND: Drilling firmFRED: H. A.	EHRhillside?			Loc In diagram below road and lot line.		



Date. 104.3.

Address MOUNT FOREST

Name of Driller FRED. H. McLuttau:

Address MOUNT FOREST

I certify that the foregoing statements of fact are true.



MINISTRY OF THE ENVIRONMENT COPY

The Ontario Water Resources Act WATER WELL RECORD

Ontario	SPACES PROVIDED ECT BOX WHERE APPLICABLE	1:	703988	1,7,0,0,4	S,R,E, O,1
COUNTY OR DISTRICT	TOWNSHIP, BOROUGH, CITY, TOWN		_	· KIE	237
	victh	04		· ~ L	DAY 17 NO 8 YR 89
	٧G	RC.	ELEVATION RC.	BASIN CODE	II IV
1 2 10 12	17 18	24 25	26 30	31	47
L(OG OF OVERBURDEN AND			RAL DESCRIPTION	DEPTH - FEET
GENERAL COLOUR COMMON MATERIAL	OTHER MATERIAL				0 - 5/
Clay 25+	ones grow			,	5/-
Limeston	,				51 - 150
31		444			
32	51 CASING & OPE	N HOLE BEG		ZE(S) OF OPENING	65 75 BO 31-33 DIAMETER 34-38 LENGTH 39-40
WATER RECORD WATER FOUND KIND OF WATER	INSIDE WATERIAL THIS	ALL DEPT	'H - FEET	ATERIAL AND TYPE	1NCHES FEET DEPTH TO TOP
10-13 1 FRESH 3 SULPHUR 14 MINERALS	INCHES IN	CHES FROM	- 5 313-16		OF SCREEN
15-18 1 FRESH 3 SULPHUR 19 4 MINERALS	2 GALVANIZED 3 CONCRETE 4 OPPEN HOLE 5 PLASTIC	88 43	150 61		G & SEALING RECORD
2 SALTY 6 GAS 20-23 1 FRESH 3 SULPHUR 4 MINERALS	17-18 1 DSTEEL 2 DGALVANIZED	38	20-23 DEP	то то	MATERIAL AND TYPE (CEMENT GROUT LEAD PACKER, ETC.)
25-28	3 □ CONCRETE 4 □ □ □ □ □ □ HOLE 5 □ □ □ PLASTIC 24-25 26		27-30	10-13 14-17	
2 SALTY 4 MINERALS 6 GAS 30-33 1 FRESH 4 SUPPLIER 34	1 DSTEEL 2 DGALVANIZED 3 DCONCRETE			26-29 30-33 80	
z SALTY 6 GAS	4 □ OPEN HOLE 5 □ PLASTIC TE N-14 DURATION OF PUMPIN				S WELL
71 PUMPING TEST METHOD COM P. PUMPING RI	7 GPM 43 HOURS	17-18		LOCATION C	ES OF WELL FROM ROAD AND
LEVEL PUMPING	LEVELS DURING 1 PUM 2 RECO	OVERY	IN DIAGRAM I LOT LINE	INDICATE NORTH BY A	RROW.
	29-31 32-34	60 MINUTES 35-37 126 FEET			
22 FEET 93 FEET 126 IF FLOWING. 38-41 PUMP INTAI	E SET AT WATER AT END OF TE	ST 42			
IF FLOWING. GIVE RATE RECOMMENDED PUMP TYPE	70 FEET	45-49			
SHALLOW DEEP SETTING	146 FEET RATE 7	GPM	>	<u>ب</u> ا	
FINAL 54 WATER SUPPLY	S ABANDONED, INSUFFICE		110001		
STATUS 3 DESTRUCTION OF WELL 4 RECHARGE WELL	7 UNFINISHED	LITY	7		
55-56 1 to DOMESTIC	5 COMMERCIAL 6 MUNICIPAL		()	0',	
WATER 2 STOCK 3 IRRIGATION 4 INDUSTRIAL	7 DUBLIC SUPPLY COOLING OR AIR CONDITION	IING		33 7	
OTHER	9 NOT USE	i.b		71	
METHOD CABLE TOOL					67206
OF CONSTRUCTION To a contact the contact of the co	9 DRIVING	OTHER	DRILLERS REMARKS		01200
NAME OF WELL CONTRACTOR	WELL CO	NTRACTOR'S NUMBER	DATA	51 3 8 1 3°	DATE RECEIVED 63-68 80
S, Neuman	dalk out.		SOURCE DATE OF INSPECTION	INSPECTOR	SEP 0 7 1989
NAME OF WELL TECHNICIAN	dalk out.	CHNICIAN'S	REMARKS		
ADDRESS ADD	nn $T-c$	2 14	OFFICE		CSS.ES
y. Naman	1	YR	Ö		FORM NO. 0506 (11/86) FORM



The Ontario Water Resources Act WATER WELL RECORD

Ontario 1. PRINT ONLY IN SPACES PROVIDED 2. CHECK S CORRECT BOX WHERE APPLICABLE 1 2	704374 NUNICIP O 4 SIRVEY FIG. LOT 25-27
COUNTY OR DISTRICT TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE	CON: 1 FAST 241
(1) \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	DAY O NO YR.
NG RC.	ELEVATION RC BASIN CODE II III IV
LOG OF OVERBURDEN AND BEDROC	K MATERIALS (SEE INSTRUCTIONS)
CONTRAL COLOUR MOST OTHER MATERIALS	GENERAL DESCRIPTION FROM TO
CARY GRALEL STONES	3-19
CARY GRAVEL	9-40
LIMIETONE	40+102
31	
32 10 14 15 21 21 32 44 14 15 16 17 17 18 18 18 18 18 18 18 18 18 18 18 18 18	43 54 55 75 80 ECORD SIZE(S) OF OPENING 31-33 DIAMETER 34-38 LENGTH 39-40
WATER FOUND KIND OF WATER INSIDE WATERIAL THICKNESS	EPTH - FEET INCHES FEET OF MATERIAL AND TYPE DEPTH TO TOP 41-44 30
10-13 FRESH 3 CISULPHUR 14 INCHES INCHES PRO	OF SCREEN FEET
15-18 1 FRESH 3 USULPHUR 19 4 UNINERALS 4 OPEN HOLE 5 OPEN HOLE 5 OPEN HOLE	61 PLUGGING & SEALING RECORD
20-23 1 FRESH 3 SULPHUR 24 17-18 1 STEEL 2 GALVANIZED	20-23 DEPTH SET AT - FEET MATERIAL AND TYPE (CEMENT GROUT FROM TO LEAD PACKER EIC)
25-28 1 FRESH 3 SUMPPLIE 29 PEASTIC 26	27-30 18-21 22-25
2 SALTY 6 GAS STEEL 2 GANCETE 30-33 1 FRESH 4 MINERALS 4 OPEN HOLE	26-29 30-33 80
Z SALTY 6 GAS 5 DPLASTIC	LOCATION OF WELL
71 PUMPING TERMETHOD 10 PUMPING RATE 11-14 DURATION OF PUMPING 17-18 10-16 PUMPING 17-18 HOURS MINS	IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND
STATIC END OF WATER LEVELS DURING 2 RECOVERY	LOT LINE INDICATE NORTH BY ARROW.
12 / 2 / 5 / 5 / 5 / 5 / 5 / 5 / 5 / 5 /	DUNDALK
IF FLOWING. GIVE RATE GPM RECOMMENDED PUMP TYPE RECOMMENDED PUMP RECOMMENDED	
RECOMMENDED PUMP TYPE RECOMMENDED 43-45 RECOMMENDED PUMP PUMP PUMP PUMP PUMP PUMP POMPING POMP	
SO-53	
FINAL WATER SUPPLY B ABANDONED, INSUFFICIENT SUPPLY B ABANDONED POOR QUALITY	S.R. 240
STATUS 5 TEST HOLE 7 UNFINISHED OF WELL 4 RECHARGE WELL DEWATERING	9
S5-56 1 DOMESTIC 5 COMMERCIAL 2 STOCK 6 MUNICIPAL WATER 5 IRRIGATION 7 PUBLIC SUPPLY	کا کی ا
USE 1 IRRIGATION 7 PUBLIC SUPPLY 1 INDUSTRIAL 0 COOLING OR AIR CONDITIONING 0 OTHER 9 NOT USED	311 /
57 CABLE TOOL 6 BORING	
METHOD OF ROTARY (CONVENTIONAL) POTARY (REVERSE) ORIGINAL OF OF OF OF OF OF OF OF OF O	86241
5 THE PERCUSSION UDIGGING UDTHER	DRILLERS REMARKS DATA S8 CONTRACTOR \$62 DATE RECEIVED 63-68 80
LIESCO UMES	≥ SOURCE 3813 OCT 21 1991
BR# 4 DUNDALK	я ш
NAME OF WELL TECHNICIAN NAME OF WELL TECHNICIAN SIGNATURE OF TECHNICIAN/CONTRACTOR SUBMISSION DATE	N REMARKS
SIGNATURE OF TECHNICIAN/CONTRACTOR SUBMISSION DATE J. V QUM QVM. DAYNOYR	CSS.ES
JI VI KUVIII I III	FORM NO. 0506 (11/86) FORM 9

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ev. 5 R 1685 WATED WATER			1		· 4
asin 2 3 WATER WE	LL	REC	UKD	Prot	2
County or District	Town	ship, Village,	Town or City	male	Hon
Con/, S.W. T. S.R. 80 Lot 237-239-240	Date	completed	/ O	apri	1969
	ldre	ess Box	227	Dem	of a lo
Casing and Screen Record					View Vi
Inside diameter of casing 4"	St	atic level	4 3	ng Test	
Total length of casing 40					G.P.M.
Type of screen		imping level		7	G.P.M.
Length of screen	- 1	uration of test		_ /	4
Depth to top of screen	w	ater clear or cl	oudy at end of	test Clor	ede
Diameter of finished hole 4"	Re	ecommended	pumping rate	351/	O T CRM
	wi	th pump settir	ng of 35	feet be	low ground surface
Well Log				T	er Record
Overburden and Bedrock Record		From ft.	To ft.	Depth(s) at which water(s	Kind of water (fresh, salty,
Topsoil		0	7	found	sulphur)
De la De					
Tana o Chay			15	10	
Fravel & Boulders.		15	40	60	Facel
Hart Hand Bank		// 6		to	"Estan.
Hard Ley Poch		70	66	66	
				00	
For what purpose(s) is the water to be used?			I nontin	C 344 II	
Household & Stock		In diagram	Location of the below shown	or we ll distances of we	all from
Is well on upland, in valley, or on hillsigle? Holanda		road and	lot line. Indi	cate north by	arrow.
Drilling or Boring Firm Less ham Drilling					
Enterprises Limited of		/			
Address 29%					
Dusham Unt		1		1	
Licence Number 32931	N			L	_
Name of Driller or Bore Col Hotels Riss	7	·		-	
Address Duthan Ont		Wel	1 0 /14	O ROD,	
Date Dept 1969	′			1	
(Signature of Licensed Drilling or Boring Contractor)			7	i o	×
Form 7				3	
OWRC COPY			-		
			:88.8	>	

MINISTRY OF THE ENVIRONMENT The Ontar - Water Resources Act ER WELL RECORD 2. CHECK 🗵 CORRECT BOX WHERE APPLICABLE OUNTY OR DISTRICT 6 rey DATE COMPLETED LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS) MOST GENERAL COLOUR OTHER MATERIALS COMMON MATERIAL GENERAL DESCRIPTION Top soil LILLOS ton

0002 62 1 0040 051211 0048 11 1 9190 15

MATERIAL

2 GALVANIZED
3 CONCRETE

DOMEN HOLE

Z GALVANIZED

CONCRETE
OPEN HOLE

2 GALVANIZED

FEET RATE 0010

5 ABANDONED, INSUFFICIENT SUPPLY 6 ABANDONED, POOR QUALITY

9 NOT USED

15-16 HOURS

32-34

PUMPING PECOVERY

2 CLOUDY

46-4

GPM

OFFICE USE ONLY

3 CONCRETE

1 T STEEL

1 D STEEL

51

Ø5 .;°

WATER LEVELS DURING 15 MINUTES | 30 MINUTES 26-28

FEET 06

PM FEE
RECOMMENDED 43-45
PUMP
SETTING 75 FEET
GPM./FT. SPECIFIC CAPACITY

FEET

FEET

→ □ UNFINISHED

PUBLIC SUPPLY

. COOLING OR AIR CONDITIONING

6 D BORING

a

JETTING

6 MUNICIPAL

41

WATER RECORD

FRESH 3 SULPHUR
2 SALTY 4 MINERAL

1 FRESH 3 SULPHUR
2 SALTY 4 MINERAL

1 FRESH 3 SULPHUR

FRESH 3 SULPHUR

BATEER

WATER LEVEL END OF PUMPING

4 MINERAL

3 SULPHUR

1 X WATER SUPPLY
2 OBSERVATION WELL

4 RECHARGE WELL

☐ OTHER

ROTARY (AIR) 5 AIR PERCUSSION

2 ROTARY (CONVENTIONAL)
3 ROTARY (REVERSE)

MINISTRY OF THE ENVIRONMENT COPY

1 2 DOMESTIC 2 ☐ STOCK

3 | IRRIGATION

4 | INDUSTRIAL

LE CABLE TOOL

4 🗆

2 🗌 SALTY

1 | FRESH 2 | SALTY

Z SALTY

RECOMMENDED PUMP TYPE

FINAL

STATUS

OF WELL

WATER

METHOD /

OF

DRILLING

NTRACTOR

USE O

☐ SHALLOW DEEP

-0048

100

0/00

3. Dundalk

SCREEN

61

FROM

18-2

26-25

MATERIAL AND TYPE

- FEET

22-2

30-33

LOCATION OF WELL

IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND LOT LINE. INDICATE NORTH BY ARROW.

N.

38/3

Chanard From 17021225

CASING & OPEN HOLE RECORD

188

FROM

40-

48

DEPTH TO TOP

PLUGGING & SEALING RECORD

MATERIAL AND TYPE

DATE RESTIVE 5 07 76

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WI

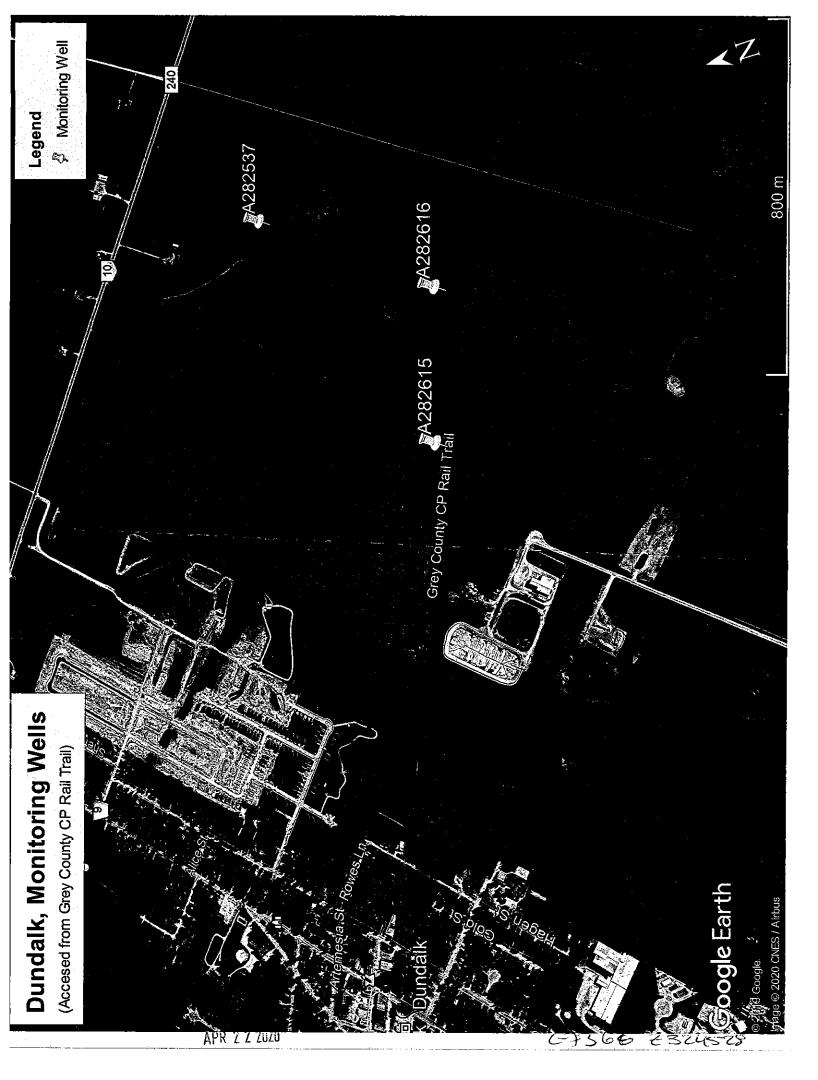
FORM 7 MOE 07-091

2

4/8

Onta	Ministry of the Environm and Climate Change	nent Well T	ag No. (Place St	icker and/c	or Print Below)		- 000			ecord
Measurements re	ecorded in: Metric Imper	ial AI=	16434			Keguiatioi 	1 903 (Page	4	ources Act
Well Owner's	Information Last Name / Organ	nization			E-mail Address			T		
	LYSTEK	nzanon			L-man Address				by We	Constructed ell Owner
	Street Number/Name)		Municipality		Province 00	Postal Code				area code)
Well Location	-PARKWAY		DINDAK		012	NOICH	DIU	2111	112101	<u> </u>
<u></u> .	ocation (Street Number/Name)		Township			Lot		Concession	on	
County/District/Mi	PARKWAY unicipality		City/Town/Village				Provir	ice	Postal	Code
_		MWWW.	DUNDACK	10.11.41			Ont		No	CIBO
UTM Coordinates	Zone Easting Northing Northing		Municipal Plan an	d Sublot N	umber		Other			
Overburden and	d Bedrock Materials/Abandonme		ord (see instruction	s on the bac	ck of this form)				Dan	No. (no. (ft)
General Colour	Most Common Material	0	ther Materials		Gene	ral Description			From	th (<i>m/ft</i>)
Braun	SAND	6RA	NEC		LOOSE				0	45
VALADORI PROPERTY AND A STATE OF THE STATE O							***************************************			
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			ALFALAMORPH HIRE PARTER							
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					Andrews Control of the Control of th					
	Annular Spa	ce				Results of W	ell Yie	ld Testing	J	
Depth Set at m	(Mg) Type of Sealant U	Jsed	Volume Plac		fter test of well yield, ☐ Clear and sand f			aw Down		ecovery Water Level
0 0.9	,		121	· · ·	Other, specify		(min)	(m/ft)	(min)	(m/ft)
0.9 4.5		<u> </u>	4)	Tif	pumping discontinue	ed, give reason:	Static Level			
0.1) · Z SAND						1		1	
				Pi	ump intake set at (/	n/ft)	2		2	
Mothod of	f Construction	Well U	Isa	PI	umping rate (l/min /	GPM)	3		3	
Cable Tool	☐ Diamond ☐ Public	☐ Comm	nercial		uration of pumping		4		4	
Rotary (Convent				atering		nin	5		5	
☐ Boring ☐ Air percussion	☐ Digging ☐ Irrigation ☐ Industria		ng & Air Conditioning	Fi	nal water level end o	of pumping (m/ft)	10		10	
Other, specify	Other, sp			If	flowing give rate (//	min / GPM)	15		15	
	Construction Record - Casing	Depth (m)ft)	Status of V			- double (m/fil	20		20	
Djameter (Galv	n Hole OR Material Wall vanized, Fibreglass, Thickness crete, Plastic, Steel) (cm/in) F	rom To	☐ Water Suppl☐ Replacemen	1 1 1	ecommended pum	p deptii (<i>iiwit)</i>	25		25	
) 1.5	☐ Test Hole ☐ Recharge W		ecommended pum min / GPM)	p rate	30		30	
5.7 P	LASTIC C	/ (.5	☐ Dewatering \ ✓ Observation :	Well	/ell production (l/mir	- (CDM)	40		40	
			Monitoring Ho	ole 📗		17 Gr WJ	50		50	
			(Construction	1) Hay	isinfected? Yes No		60		60	
	Construction Record - Screen		Insufficient S	Supply -		Map of W	ell Lo	cation		
Outside Diameter	Material Slot No.	Depth (m)ti)	Water Qualit	y P	lease provide a map	below following	inetrue	tions on the	-back	\
Diameter (Plasti	ic, Galvanized, Steel)	rom To	specify	Other,		_ ('	alacon)		`\
6 PL	ASTIC ID 1	5 45	Other, speci	fy	1	7		4		7
		Desiring the second sec			LAGOON	, /		N		7
Water found at D	Water Details epth Kind of Water: Fresh Un	itested De	Hole Diameter epth (m/ft) Dia	meter			bonness			
(m/ft)	Gas Other, specify	From	To (6	em)in)	1	-			+ T2	~
	epth Kind of Water: Fresh Un Gas Other, specify	itested O	4.5	5	191				1	
(m/ft) U	repth Kind of Water: Fresh Ur	itested							1	
(m/ft)	Gas Other, specify			0.033500070000			33	-	1	
Business Name of	Well Contractor and Well Tecl f Well Contractor		nation Well Contractor's Lice	nce No.	1		2		ļ	
CHT D	RILLING INC.			6		ECO PHE	CWA	1		
	(Street Number/Name)	3	Municipality WATERLOO	c	omments:					
Province	Postal Code Business E-m	ail Address	VVI 1 (CI-SACO)	- I leave						
No.	NOBZMO CUT INC. (inc. area code) Name of Well Techn	NET	e. First Name)	in	formation	Package Deliver		Min Audit No.	istry Use Z 2 N	300 only 7158
15/19/699	9151715 BLACK, CH	RS			ackage y y y y Date \	│	ם ם		- <u>L</u> U	1 T O O
Well Technician's Lic	cence No. Signature of Technician and		Date Submitted	1 H///	Yes	1504		RMAX	112	015
				ــاكــــــ			- II	LEAD OF WALL	AND THE PERSONS	

C	Ontario		y of the Env		Well Ta	g No. (Pla	ace Sticker a	nd/or Print Be	·	- 002 6	_		Record
Меаѕигел	ments recor	ded in: 🔽	Metric □	Imperial	A28	2537	-		Regulation	n 903 C	ntario vi Pag		ources Act
Well Ow First Name	vner's Info e	国际的时间的时间的时间的 医前部的	Last Name /	Organizatio				E-mail A	ddress	19 +		☐ Weli (Constructed
Mailing Ad	ddress (Stree	t Number/Nar	ToU,	whip of	1 Sow	Municipality		Province	Postal Code	e	Telephon	by We	ell Owner
	67 (rey Co	•	road	၅	Du.	Malk	٩٥	MO C /	No.			#11p
Valentin March 1, March 1, 1974 also 19	Headon Cantagon (1994)	on (Street Nu	mber/Name)		[7	ownship			Lot		Concess	ion	
1530 County/Dis	<u>m Se</u> istrict/Municip	oality	ones ci	<u>v</u> 59	om N	E oF City/Town/V	GREN Co	STY CF	PAIL TRAIL	 Provir	nce	Postal	Code
UTM Coor	rdinates Zon	e . Easting	N	orthing		DUNDA Aunicipal P	ا lan and Sublo	of Number		Ont	ario		
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General C			ials/Abande mon Material			o rd (see ins ner Material		e back of this fo	rm) General Description	<u> </u>		Dep From	th ((1)/10
BLACK	٠ -	70Psoil				•						0	0.6
BROW		SIAND		5	uct Gra	<u>lauel</u>						0.6	
GREU	1 5	jut Tu	<u>L</u>				·		DENISE			36	5.1
			-										
											-		
	l		Annular	Control of the Control of the Control					Results of W	/ell Yiel	d Testin		
Depth S From	Set at (m/ft)		Type of Sea (Material ar				ne Placed	∐ Clear an		Time	aw Down Water Le		ecovery Water Level
0	1-8		HOLFRI	رکاد	·	41		☐ Other, s _i	scontinued, give reason:	(min) Static	(m/ft)	(min)	(m/ft)
1.8	5.1	#2 5	AND			41			, V	Level 1		1	
		<u> </u>			-			Pump intake	set at (m/ft)	2		2	
Met	thod of Co	nstruction			Well Us	e		Pumping rate	(Vmin / GPM)	3		3	
Cable To	ool (Conventional)	☐ Diamond	1 =	blic mestic	☐ Commer	_	Not used Dewatering	Duration of pu	ımping	4		4	
☐ Rotary (F	,	☑ Driving ☐ Digging	I =	estock	Test Hole	_	 ✓ Monitoring	hrs +	min wel end of pumping <i>(m/ft</i>)	5		5	
☐ Air percu☐ Other, sa			Ind	lustrial ner, <i>specify</i>						10		10	
		struction R	1			+=	s of Well		rate (l/min / GPM)	20		20	
Inside Diameter (cm/in)	(Galvanize	e OR Material d, Fibreglass, Plastic, Steel)	Wall Thickness (cm/in)	From	th (77 9) To	1 = '	ement Well	Recommende	ed pump depth (m/ft)	25		25	
254	PLAST	١Ç		0	21	☐ Test He	rge Well	Recommende (I/min / GPM)	ed pump rate	30		30	
						☑/Observ	ering Well vation and/or ring Hole	Well production	on (Vmin / GPM)	40		40	
	ļ	<u>-</u>		_		☐ Alterati	· ·	Disinfected?		50		50	
	Cor	struction R	ecord - Scr	een	Maria de Servico	d	cient Supply	Yes _	No Map of W	60	ation	60	Digwedhay V
Outside Diameter	Ma	iterial vanized, Steel)	Slot No.	Dept	հ (Թ)	Water	oned, Poor Quality oned, other,	Please provid	de a map below followi			the back.	
(cm/in)	 		10	From	To	specify							
<u>33</u>	PLAST	<u></u>	10	2.1	5.1	Other,	specify						
		Water Det	and the recognition and Section 19 contact and	18 Ednavier	and the state of the state of	ole Diame	ter.	1	SEE ATT	rach	60 1	JAP	
		Kind of Water Other, spe		Untested	Depti From	To	Diameter (cm)n)						
		Kind of Water		Untested	0_	5.1	10						
Water found	nd at Depth	Kind of Water	: Fresh	Untested	 		 						
- (m	rivit) [_]Gas[Other, spe	<u></u>		<u> </u>								
(CMT D	rilling Ir	nc.	T ice	ense No	7266	Licence No.						
		dustrial						Comments:					
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	519-699			Black,	_		:	Well owner's information package	Date Package Deliver		Mini Audit No.	stry Use Z3 2 <u>/</u>	528
	3711 <u> </u>	<u> </u>	<u> </u>		2020 0	1122		delivered Yes	Date Work Completed	שוט	Ann	-UZ4	
To	ech License	No. Signatu	re of Contrac	tor	Date Su	bmitted	D D M	☐ No	202061	bb	Received 1	Z 2 2020	Out = 0515
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Map: Well records

This map allows you to search and view well record information from reported wells in Ontario.

Full dataset is available in the Open Data catalogue (https://data.ontario.ca/dataset/well-records).

Go Back to Map

Well ID

Well ID Number: 7357122 Well Audit Number: *Z324529* Well Tag Number: *A282216*

This table contains information from the original well record and any subsequent updates.

Well Location

Address of Well Location	
Township	DUNDALK VILLAGE
Lot	
Concession	

County/District/Municipality	GREY
City/Town/Village	DUNDALK
Province	ON
Postal Code	n/a
UTM Coordinates	NAD83 — Zone 17 Easting: 550215.00 Northing: 4890067.00
Municipal Plan and Sublot Number	
Other	

Overburden and Bedrock Materials Interval

General Colour	Most Common Material	Other Materials	General Description	Depth From	Depth To
BLCK	LOAM			0 m	.6 m
BRWN	SAND	SILT	GRVL	.6 m	3.6 m
GREY	SILT	TILL		3.6 m	4.5 m

Annular Space/Abandonment Sealing Record

Depth From	Depth To	Type of Sealant Used (Material and Type)	Volume Placed
0 m	1.2 m	3/8 HOLEPLUG	
1.2 m	4.5 m	#2 SAND	

Method of Construction & Well Use

Method of Construction	Well Use
Driving	
	Monitoring

Status of Well

Observation Wells

Construction Record - Casing

Inside Diameter	Open Hole or material	Depth From	Depth To
2.54 cm	PLASTIC	0 m	1.5 m

Construction Record - Screen

Outside Diameter	Material	Depth From	Depth To
3.3 cm	PLASTIC	1.5 m	4.5 m

Well Contractor and Well Technician Information

Well Contractor's Licence Number: 7366

Results of Well Yield Testing

After test of well yield, water was	
If pumping discontinued, give reason	
Pump intake set at	
Pumping Rate	
Duration of Pumping	
Final water level	
If flowing give rate	
Recommended pump depth	
Recommended pump rate	

Draw Down & Recovery

Draw Down Water level	Recovery Time(min)	Recovery Water level
	1	
	2	
	3	
	4	
	5	
	10	
	15	
	20	
	25	
		Water level Time(min) 1 2 3 4 5 10 15 20

30	30	
40	40	
45	45	
50	50	
60	60	

Water Details

Water Found at Depth	Kind

Hole Diameter

Depth From	Depth To	Diameter
0 m	4.5 m	10 cm

Audit Number: Z324529

Date Well Completed: January 14, 2020

Date Well Record Received by MOE: April 22, 2020

Related

How to use a Ministry of the Environment map (https://www.ontario.ca/page/how-use-ministry-environment-map#wells)

Technical documentation: Metadata record (https://data.ontario.ca/dataset/well-records/resource/3031344e-e3f2-48d5-888c-c1deadfd2f77)

Updated: October 18, 2021 Published: March 20, 2014

Map: Well records

This map allows you to search and view well record information from reported wells in Ontario.

Full dataset is available in the Open Data catalogue (https://data.ontario.ca/dataset/well-records).

Go Back to Map

Well ID

Well ID Number: 7357123 Well Audit Number: *Z324530* Well Tag Number: *A282615*

This table contains information from the original well record and any subsequent updates.

Well Location

Address of Well Location	
Township	DUNDALK VILLAGE
Lot	
Concession	

County/District/Municipality	GREY
City/Town/Village	DUNDALK
Province	ON
Postal Code	n/a
UTM Coordinates	NAD83 — Zone 17 Easting: 549836.00 Northing: 4890177.00
Municipal Plan and Sublot Number	
Other	

Overburden and Bedrock Materials Interval

General Colour	Most Common Material	Other Materials	General Description	Depth From	Depth To
BRWN			FILL	0 m	1.67 m
BLCK				1.67 m	2.3 m
BRWN	SILT		SNDY	2.3 m	2.6 m

BRWN	SAND			2.6 m	3.6 m
GREY	SILT	TILL	DNSE	3.6 m	6.1 m

Annular Space/Abandonment Sealing Record

Depth From	Depth To	Type of Sealant Used (Material and Type)	Volume Placed
0 m	2.4 m	3/8 HOLEPLUG	
2.4 m	6.1 m	#2 SAND	

Method of Construction & Well Use

Method of Construction	Well Use
Driving	
	Monitoring

Status of Well

Observation Wells

Construction Record - Casing

|--|

2.54 cm	PLASTIC	0 m	3 m

Construction Record - Screen

Outside Diameter	Material	Depth From	Depth To
3.3 cm	PLASTIC	3 m	6.1 m

Well Contractor and Well Technician Information

Well Contractor's Licence Number: 7366

Results of Well Yield Testing

After test of well yield, water was	
If pumping discontinued, give reason	
Pump intake set at	
Pumping Rate	
Duration of Pumping	
Final water level	

If flowing give rate	
Recommended pump depth	
Recommended pump rate	
Well Production	
Disinfected?	

Draw Down & Recovery

Draw Down Time(min)	Draw Down Water level	Recovery Time(min)	Recovery Water level
SWL			
1		1	
2		2	
3		3	
4		4	
5		5	
10		10	

15	15	
20	20	
25	25	
30	30	
40	40	
45	45	
50	50	
60	60	

Water Details

Wa	ter Found at Depth	Kind

Hole Diameter

5	5 4	
Depth	Depth	Diameter

F	From	То	
(0 m	6.1 m	10 cm

Audit Number: Z324530

Date Well Completed: January 14, 2020

Date Well Record Received by MOE: April 22, 2020

Related

How to use a Ministry of the Environment map (https://www.ontario.ca/page/how-use-ministry-environment-map#wells)

Technical documentation: Metadata record (https://data.ontario.ca/dataset/well-records/resource/3031344e-e3f2-48d5-888c-c1deadfd2f77)

Updated: October 18, 2021
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